

Geomechanical rock properties of a basaltic volcano

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In volcanic regions, reliable estimates of mechanical properties for specific volcanic events such as cyclic inflation-deflation cycles by magmatic intrusions, thermal stressing, and high temperatures are crucial for building accurate models of volcanic phenomena. This study focuses on the challenge of characterizing volcanic materials for the numerical analyses of such events. To do this, we evaluated the physical (porosity, permeability) and mechanical (strength) properties of basaltic rocks at Pacaya Volcano (Guatemala) through a variety of laboratory experiments, including: room temperature, high temperature (935°C), and cyclically-loaded uniaxial compressive strength tests on as-collected and thermally-treated rock samples. Knowledge of the material response to such varied stressing conditions is necessary to analyze potential hazards at Pacaya, whose persistent activity has led to 13 evacuations of towns near the volcano since 1987. The rocks show a non-linear relationship between permeability and porosity, which relates to the importance of the crack network connecting the vesicles in these rocks. Here we show that strength not only decreases with porosity and permeability, but also with prolonged stressing (i.e., at lower strain rates) and upon cooling. Complimentary tests in which cyclic episodes of thermal or load stressing showed no systematic weakening of the material on the scale of our experiments. Most importantly, we show the extremely heterogeneous nature of volcanic edifices that arise from differences in porosity and permeability of the local lithologies, the limited lateral extent of lava flows, and the scars of previous collapse events. Input of these process-specific rock behaviors into slope stability and deformation models can change the resultant hazard analysis. We anticipate that an increased parameterization of rock properties will improve mitigation power.

Keywords: volcanic materials, mechanical properties, high temperature, strength, Pacaya volcano

Introduction

Several studies of non-volcanic slopes using numerical models and geomechanical material properties have been adapted for the investigation of volcanic collapses, slope stability, and ground deformation. These include applying Limit Equilibrium Methods (LEM) (Voight et al., 1983; Iverson, 1995; Elsworth and Voight, 1996; Hürlimann et al., 2000; Reid et al., 2000; Donnadieu et al., 2001; Okubo, 2004; Apuani and Corazzato, 2010) and numerical modeling by Distinct Element Methods (DEM) (Zimbelman et al., 2000; Apuani et al., 2005a,b, 2007; Apuani and Corazzato, 2010) and Finite Element Methods (DEM) (Hürlimann et al., 2004), Finite Difference Methods (FDM) (Hürlimann et al., 2000; Apuani et al., 2005a,b, 2007; Apuani and Corazzato, 2010) and Finite Element Methods

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(FEM) (Sousa and Voight, 1995; Apuani and Corazzato, 2010; del Potro et al., 2013; Schaefer et al., 2013). However, numerical data on the relevant mechanical properties for volcanic rocks and rock masses used in these types of studies remain a major source of uncertainty. Quantifying these properties is challenging due to the diversity of materials that compose volcanoes, the unknown interior structures and geometry, and the often inaccessible locations of volcanic rock masses (Thomas et al., 2004; del Potro and Hürlimann, 2008).

Experimental rock mechanics has traditionally been executed in the fields of civil engineering and mining as well as structural geology, with a focus on sedimentary and intrusive igneous rock (see Paterson and Wong, 2005 and references therein). Early efforts were made to understand rock behavior and strength under high pressure (Scholz, 1968; Tapponnier and Brace, 1976; Batzle et al., 1980), high temperature (Griggs et al., 1960; Simmons and Cooper, 1978; Handin and Carter, 1979; Fredrich and Wong, 1986), and in water saturated conditions (Bauer et al., 1981). Additional work was done to connect mineralogy and microstructures with rock strength (Merriam et al., 1970; Přikryl, 2001) and to model the mechanical properties of porous material; that is, the poreemanated crack model (Sammis and Ashby, 1986) and the wing-crack model (Ashby and Sammis, 1990). Only recently have laboratory studies begun to systematically investigate the microstructural, physical, and mechanical properties of volcanic rocks, a material infamously known for its formation under disequilibrium conditions, thereby rich in heterogeneities at all scales. These include experiments on a range of material under relevant volcanic conditions such as thermal stressing (Vinciguerra et al., 2005; Kendrick et al., 2013a; Heap et al., 2014a), cyclic inflation-deflation cycles by intrusions (Heap et al., 2009, 2010; Kendrick et al., 2013a), fragmentation (Spieler et al., 2004; Kueppers et al., 2006; Scheu et al., 2008), and flow or fracture at high temperatures and/or pressures (Balme et al., 2004; Rocchi et al., 2004; Smith et al., 2005; Lavallée et al., 2007, 2008; Benson et al., 2008; Cordonnier et al., 2009; Loaiza et al., 2012; Kendrick et al., 2013b).

These investigations have shown that porosity is a fundamental control on rock properties. At volcanoes, the porous network comprises of vesicles and fractures that build the permeable network and which controls degassing (Ashwell et al., 2015). Porosity also controls the mechanical response of materials whereby their strength decreases and elasticity increases with porosity (e.g., Al-Harthi et al., 1999; Spieler et al., 2004; Paterson and Wong, 2005; Heap et al., 2014a,b). Thermal stressing of rock has also been argued to affect its strength, although damage imparted by thermal cracking has been shown to lead to unsystematic changes in mechanical response (Jones et al., 1997; Rocchi et al., 2003; Balme et al., 2004; Kendrick et al., 2013a; Heap et al., 2014a); the ambiguity of this claim has been linked to the extent of pre-existing microcrack damage in a rock (Vinciguerra et al., 2005; Heap et al., 2014a). Other rocks can debilitate upon thermal stressing, as is the case during decarbonation (Heap et al., 2013b) and dehydroxylization (Heap et al., 2012) of certain rock-forming minerals. Cyclic loading can lead to mechanical weakening (Kendrick et al., 2013a) and an increase in heterogeneity of elastic modulus values, likely due to an increase in anisotropic crack damage within the samples (Heap and Faulkner, 2008; Heap et al., 2010).

This paper focuses on the challenge of characterizing volcanic materials for numerical analyses and addressing the scarcity of published geomechanical data for volcanic materials. To do this, we evaluated the mechanical properties of rocks at Pacaya Volcano (Guatemala) using a variety of laboratory experiments, including room temperature, high temperature, and cyclicallyloaded uniaxial compressive strength tests (UCS) on as-collected and thermally treated rock samples. Pacaya is an ideal case study for material characterization for use in deformation analysis due to its persistent activity, significant morphological changes, and history of slope instability (Vallance et al., 1995; Matías Gómez et al., 2012; Schaefer et al., 2013). By considering the non-unique geomechanical behaviors when the materials are subjected to this wide range of stressing conditions, we may allow the determination of more encompassing and realistic models that will help better predict volcanic processes and imminent catastrophes.

Geologic Setting

Pacaya is a basaltic stratovolcano complex in the Central American Volcanic Arc (Figure 1A) that has been persistently active since 1961 with over 250 lava flows, intermittent strombolian activity, and ash and fumarolic plumes (Matías Gómez et al., 2012; Rose et al., 2013). Between 0.6 and 1.5 ka B.P., the SW sector of the initial cone failed in a major edifice collapse, forming a debris avalanche that traveled 25 km SW, containing approximately 0.65 km³ of debris (Kitamura and Matías Gomez, 1995; Vallance et al., 1995). The edifice re-built within the collapse amphitheater until the nineteenth century, when it went into a period of repose. Activity renewed once again in 1961, loading the SW flank of the edifice with 100-150 m thick of primarily effusive lava flows. Although the volcano has grown substantially, the collapse scarp is still exposed today on the north and east sides of the cone on account of the asymmetric loading of material on the SW flank.

Explosive and effusive eruptions have caused constant morphometric changes to the edifice: in 1962, an oval-shaped area, approximately 300 m by 200 m, subsided near the summit. This subsidence has since been in-filled by erupted material. In 2010, a large collapse trough 50–80 m deep and 100 m wide, extending 600 m from the summit, developed concurrently with explosive activity on May 27, 2010 (**Figure 1B**). Although the exact causes of these collapses are unknown, shallow magma systems and high growth rates leading to over-steepening of the cone have been postulated as factors affecting edifice stability (Eggers, 1983; Vallance et al., 1995; Matías Gómez, 2009; Schaefer et al., 2013). The rock characterization determined in this study can help to better understand events such as these and assess potential future intrusions, collapses, and deformation events.



FIGURE 1 | (A) Map showing the location of Pacaya in Guatemala. (B) Photograph of Pacaya taken in January 2012, showing the 2010 collapse scar and (C) map showing the location of samples

Materials and Experimental Methodology

Experimental Materials Sample Characterization

Pacaya volcano is made of diverse lithologies (lavas and volcaniclastic deposits) which vary in extent and character significantly, though non-systematically (Wunderman and Rose, 1984; Matías Gómez et al., 2012). This heterogeneity at the large scales calls for a characterization of equally diverse rock types. For this experimental study, eight sample blocks were collected from different eruptive deposits (Figure 1C, see Supplementary Image for detailed volcanological map from Matías Gómez et al., 2012), which cover a range of macroscopically representative physical properties of the edifice-forming basalt at Pacaya (Table 1). These samples, which vary in porosity, crystal size, and vesicle size distribution include: two samples north of the summit from lava flows in January and August-October of 2013; two samples of blocks from rock fall collected from on top of the 2010 deposits; two samples from lava flows associated with the May-June eruption in 2010; one sample from the brecciated top of a lava flow that occurred in February of 1974; and one sample from the (>1.5 ka B.P.) southern portion of the ancestral scarp (Figures 2A-H). Typically, basalt at Pacaya is porphyritic with olivine and plagioclase phenocrysts (45 wt%), with lesser amounts of clinopyroxene and iron-titanium oxides and plagioclase dominated microlites (Bardintzeff and Deniel, 1992). No significant petrographic differences have been found between prehistoric and modern lava flows. Each of the sample blocks was drilled to prepare a series of cylindrical cores with a chosen for this study. See the Supplementary Image for a detailed volcanological map from Matias Gómez et al. (2012) with field site locations.

TABLE 1 | Sample names and eruption date.

Sample name	Eruption date		
PAC_2013_1	August–October, 2013		
PAC_RF_2	2010–2013		
PAC_SCARP	>0.2–4 ka		
PAC_2010_2	May–June, 2010		
PAC_RF_1	2010–2013		
PAC_2013_2	January, 2013		
PAC_2010_1	May–June, 2010		
PAC_1974	February, 1974		

diameter of 26 mm and a length of 52 mm to maintain a lengthto-diameter ratio of 2:1. Although other samples were collected in the field (see Supplementary Image), the eight sample blocks were chosen from which cores could be drilled to provide the widest range of representative samples while ensuring reliable and repeatable measurements of strength. Based on the abundance of effusive products, and the similarity of eruptive product composition over time, we feel as though our samples represent the majority of edifice-building material variability.

Porosity and Permeability

Before each test, porosity was determined for each core using a 100cc helium pycnometer from Micromeritics, providing measurements with an accuracy of 0.1%. First, the density of a cylindrical core (ρ_{rock}) was estimated and used to calculate the connected porosity (ϕ_c). Total porosity (ϕ_T) was also determined



FIGURE 2 | Photomicrographs of the samples chosen for this study (Left, As-collected samples; Right, Thermally stressed samples). (A) PAC_2013_1 shows intact phenocrysts and few microlites set in a glassy groundmass; (B) PAC_RF_2 is made of large plagioclase phenocrysts (broken, (Continued)

FIGURE 2 | Continued

sieved, and zoned) surrounded by fine grained groundmass; (C) PAC_SCARP contains fewer and smaller plagioclase phenocrysts surrounded by a mildly developed trachytic texture in the microcrystalline groundmass; (D) PAC_2010_2 contains well-defined plagioclase and pyroxene phenocrysts as well as large deformed pores in a fine grained groundmass host of plagioclase and iron oxides; (E) PAC_RF_1 hosts large phenocrysts of plagioclase with poikilitic inclusions and equant microlites in a fine grained groundmass rich in iron oxides; (F) PAC 2013 2 shows large phenocrysts of plagioclase and pyroxene, with some microlites in a groundmass hosting very fine grained crystals. (G) PAC 2010 1 hosts large phenocrysts, rounded pores and very fine grained groundmass. (H) PAC 1974 hosts the largest phenocrysts (of the collected samples) and a fine groundmass containing equant grains. (I-P) photomicrographs of thermally stressed samples, showing a tendency for the samples groundmass to darken due to oxidation of iron oxides and possibly micro-crystallization of the groundmass, as well as the minor development of micro-fractures.

for each sample type by measuring the density of a powdered sample (ρ_{powder}), and computing:

$$\phi_{\rm T} = (\rho_{powder} - \rho_{rock}) / \rho_{rock}$$

From this, isolated porosity was calculated by subtracting connected porosity from total porosity.

Permeability was determined for each sample block using a TinyPerm II Permeameter from New England Research Inc., which was also used to measure the permeability of 97 *in-situ* lavas and breccias (unconsolidated air-fall tephras to brecciated surfaces of lava flows) at Pacaya during fieldwork in October 2013.

Thermal Stressing

Thermal stressing occurs in volcanic edifices due to heat radiating from the magma in the conduit that may be upwards of 1100° C. Rocks in direct contact with conduit magma may experience partial melting, and further afield temperatures exceeding 900°C are likely common. Thermal analysis was performed on cores of samples in a box furnace by heating them to 1050° C and then re-cooling to room temperature at a rate of 10° C/min to assess physical changes, including oxidation and the onset of melting. This data was used to constrain the maximum experimental temperature for uniaxial compressive strength tests, as it revealed partial melting between 1000 and 1050° C for some samples; therefore a temperature of 935° C was chosen for the hightemperature tests as well as for the thermal stressing tests.

For thermal stressing, three cores from each block were placed in a box furnace and heated at a rate of 4° C/min to the target temperature, left to dwell for 1 h to allow thermal equilibration and completion of any reactions which might have taken place, and then cooled to room temperature using the same rate. A rate of 4° C/min was chosen to minimize damage that could result from temperature gradients within the sample with the presumption that heating in volcanic scenarios occurs relatively steadily, except for rocks in direct contact with magma. Porosity of samples was measured before and after thermal treatment. Thin sections were made from each block with and without thermal stressing (**Figures 2A–P**).

Experimental Methods

Uniaxial compressive strength (UCS) experiments were conducted at constant strain rates in a 100 kN Instron uniaxial loading frame (model 1362) in the Experimental Volcanology Laboratory at the University of Liverpool. A summary of the experiment type, temperature, and strain rate is shown in **Table 2**. Applied load (in kN) and displacement (mm) were measured throughout at a rate of 10–100 Hz. Following calibration, to take into account the compliance of the experimental set-up at the given conditions, axial stress (σ_a ; MPa) and strain (ε_a) were calculated. The Static Young's modulus (E_s) was calculated from the linear portion of the stress/strain curve (at 30% of the peak stress) via:

$$E_s = d\sigma_a/d\varepsilon_a$$

Room Temperature Experiments

UCS experiments were performed on dry specimens at room temperature by testing three cores (52 mm length and 26 mm diameter) from each of the eight original sample blocks as well as three thermally stressed cores from seven sample blocks. The cylindrical cores were axially loaded at strain rates of 10^{-1} , 10^{-3} , and 10^{-5} s⁻¹ (5, 0.05, and 0.0005 mm/s, respectively) until failure occurred, as defined by a sharp stress drop.

Cyclic Experiments

In cyclic experiments, the uniaxial compressive stress was applied at a constant strain rate of 10^{-5} s⁻¹ until 15% of the sample's value of UCS (as determined from previous room temperature UCS tests at a strain rate of 10^{-5} s⁻¹), then unloaded to 5% of this stress. Subsequently, a further loading/unloading cycle was imposed, but this time reaching an additional 15% load increment (i.e., 30% of UCS), then again unloaded to 5%. These cycles were repeated by increasing the peak stress in 15% stress increments of the UCS until failure. All cyclic tests were

TABLE 2 | Sample, strain rate and temperature conditions for each experiment.

Experiment I.D.	Test	Strain rate	Temperature	
		(s ⁻¹)	(° C)	
a	UCS on original cores	10 ⁻¹	20	
b	UCS on original cores	10 ⁻³	20	
с	UCS on original cores	10 ⁻⁵	20	
d	UCS on thermally stressed cores	10 ⁻¹	20	
е	UCS on thermally stressed cores	10 ⁻³	20	
f	UCS on thermally stressed cores	10 ⁻⁵	20	
g	Cyclic loading tests	10 ⁻⁵	20	
h	UCS at high temperature	10 ⁻¹	935	
i	UCS at high temperature	10 ⁻⁵	935	

conducted at room temperature on dry, as-collected specimens at a strain rate of 10^{-5} s⁻¹.

High Temperature Experiments

High temperature tests were conducted by placing a core in the same uniaxial press surrounded by a three-zone split cylinder furnace, and heated at a rate of 4°C/min until a temperature of 935 \pm 3°C was reached. Before undertaking the experimental program, the compliance of the setup at high temperature was measured and considered to remove any of the mechanical artifacts of the machine on the dataset. Samples were held to thermally equilibrate at this temperature for an hour before the experiment began. Temperature was consistently monitored during heating, cooling, and while the compressive load was applied by a thermocouple in contact with the outer surface of the specimen. Samples at high temperature were tested at strain rates of 10^{-1} and 10^{-5} s⁻¹ to determine end-member strain-rate behavior.

Results

Porosity and Permeability

Samples range from 2 to 48% total porosity, and isolated pores makes up a varying but sometimes substantial amount of total porosity (Figure 3) as a result of the varying abundance of microcracks and vesicles (Figure 2). Although porosity varied greatly between samples, it remained relatively constant between cores from the same sample, with standard deviations of <3.3% for five or more measurements. Pre- and post-thermal stressing pycnometer measurements showed only minor changes in porosity of -0.4 to +0.2% despite visible differences in sample structure that resulted from thermal cracking and/or oxidation (Figure 2). Permeability measurements showed a bimodal distribution (Figure 4A) that approximately split into coherent lavas which were sampled for this study (peak at 10⁻¹³), and brecciated lava flow surfaces and unconsolidated air-fall deposits (peak at 10^{-10}). The sample's permeability was observed to scale to porosity via the relationship for effusive lavas constrained by Mueller et al. (2005) (Figure 4B).

Uniaxial Compressive Strength Results

Here we present the results of 64 UCS experiments and the effects of (1) strain rate, (2) porosity, (3) cyclic loading, (4) thermal stressing, and (5) temperature on the strength of the tested rocks. For all UCS stress-strain curves, see Supplementary Material.

A comparison of UCS stress-strain curves for all as-collected, room temperature cores at 10^{-1} and 10^{-5} s⁻¹ strain rates is shown in **Figure 5**. These curves display the shape of typical brittle failure under compression, outlined in Heap et al. (2014a) as follows: (1) convex shape as microcracks aligned subperpendicular to the direction of loading begin to close (leading to a large strain despite little stress accumulation); (2) linear shape as the rock deforms elastically; (3) concave shape as the rock deforms inelastically and experiences strain hardening; and (4) rock failure marked by a substantial stress drop. Higher strain rate tests fail in a more brittle manner (marked by sharper curve



peaks at the stress drop) than slower strain rates (Figure 5) and reach both a higher axial strain and a higher peak stress before failure occurs (Figure 6A). Indeed, room temperature, thermally stressed, and high temperature tests all show a clear pattern of apparent strengthening with increasing strain rates, which is more significant at lower porosity (Figure 6B, Table 3). On average, room temperature cores tested at a strain rate of 10^{-1} s^{-1} failed at axial stresses 5% higher than strain rates of 10^{-3} s^{-1} , and strain rates of $10^{-3} s^{-1}$ failed 29% higher than strain rates of 10^{-5} s⁻¹. In a similar pattern, thermally stressed cores tested at a strain rate of 10^{-1} s⁻¹ failed at axial stresses 4% higher than strain rates of 10^{-3} s⁻¹, and strain rates of 10^{-3} s⁻¹ failed at a stress 22% higher than the stress at strain rates of 10^{-5} s⁻¹. Finally, high-temperature cores failed at an average 52% higher axial stress at 10^{-1} s⁻¹ as compared to strain rates of 10^{-5} s⁻¹ (Table 3).

The UCS results show that failure stress is highly dependent on porosity, with maximum stress decreasing non-linearly as porosity increases (**Figure 6B**). Samples can be divided into three porosity groups, with samples having <10% porosity averaging a UCS of 138 MPa, samples with 10–25% porosity with a strength decreasing by 55% to an average of 62 MPa, and samples with >25% porosity with a strength decreasing another 66% to an average of 21 MPa. Overall, axial compressive strength for Pacaya basalt ranges widely from 12 to 174 MPa (**Table 3**), with the lower



FIGURE 4 | (A) Permeability frequency of *in-situ* rocks at Pacaya from 97 measurements made using a portable permeameter during fieldwork in October 2013. The data shows that the permeability of lavas spans a wider range than that of breccias (defined, here as unconsolidated volcanic deposits). The samples collected for the experimental study had low relative permeability so as to constrain the strength of intact material associated with failure, such as in edifice collapse. **(B)** Permeability vs. porosity for each sample selected for this study. The best fit regression of the dataset falls within the porosity-permeability relationship for effusive lavas described by Mueller et al. (2005).

end falling into strength categories as low as chalk (Hoek, 1994). Given that the porosity of the rocks scales with permeability, we chose to assess the relationship of UCS with permeability, as this measures the strong control that cracks have on the physical properties of rocks. The data shows a remarkable linearity in UCS decrease with logarithmic increase in rock permeability (**Figure 6C**). Cyclic loading tests in which the peak load of each successive cycle was increased had a variable effect on strength (**Table 3, Figure 6**). Each cycle peak was 15% of the failure strength of the room temperature tests higher (as-collected cores at 10^{-5} s⁻¹). The highest stress was usually reached on the final cycle, exceeding all previous stress cycles (**Figures 7A,B**). However, some cyclically loaded samples failed at a lower



stress than the peak stress experienced in the penultimate cycle (**Figures 7C,D**). Compared to the mechanical behavior of the rocks during standard UCS tests at strain rates of 10^{-5} s⁻¹, four out of seven rocks failed at a lower stress during the cyclic tests, and five out of seven rocks failed at lower strain (**Figure 8A**). Continuous stress-strain curves (**Figures 7B,D**) show a degree of hysteresis, showing that at a given stress, strain is higher during unloading than during loading of a particular cycle. Each successive cycle led to further accumulated (non-recoverable) strain noted when the samples were unloaded to 5% of their value of their UCS (**Figure 8B**).

The effect of thermal stressing of cores varied across our investigated suite of samples: 12 out of 20 cores failed at a lower stress than the original samples (**Table 3**). The difference in porosity before and after thermal treatment was, however, minimal (-0.4 to +0.2%) and overall, the changes in

strength by thermal stressing are within the natural variability of each sample (**Figures 6A,B**). Additionally, there were no significant variations in the magnitude of strain to failure or style of failure upon thermal stressing (see Supplementary Information).

The deformation of rock at high-temperature exhibited an increase in strength. On average, strength at 935°C was 19 and 5% higher than the rock strength at room temperature for deformation at strain rates of 10^{-1} and 10^{-5} s⁻¹, respectively. These tests revealed that rock failure at high temperature requires a higher magnitude of strain than at room temperature; this is especially the case for the lower porosity (strongest) samples (**Figure 6A**) The peak stress of these tests also scale inversely with porosity as do the tests at room temperature (**Figure 6B**).

Almost all the samples had a Young's Modulus that scaled approximately linearly to peak strength (Figure 9A), the only



outliers were the high-temperature experiments, which had a lower Young's Modulus for a given compressive strength (**Figure 9A**). This difference becomes particularly prominent at higher strength, and corresponds to a slightly higher strain to failure, especially at low porosity (**Figure 9B**).

Discussion

Examination of the mechanical properties of eruptive products at Pacaya volcano provide us with a landscape of data from which to improve our understanding of volcano edifice properties. Of the attributes measured, porosity has the strongest control on rock strength, followed by strain rate, then temperature, which show non-negligible, second order effects. Cyclic loading and thermal stressing seem to have lesser, unsystematic influences on the mechanical behavior of the rocks tested (**Figure 6B**). The mechanical data show that the strength of the tested Pacaya basalts vary by slightly over one order of magnitude from 12 to 174 MPa across a range of porosities from 2 to 48%, corresponding to a permeability range of 10^{-12} – 10^{-15} m² for the

experimental samples. Given that the samples tested here only represent coherent rocks forming the volcano (**Figure 4A**), it is likely that incoherent lithologies such as unconsolidated breccias (not studied experimentally, but for which we measured high permeabilities in the field; **Figure 4A**) have negligible strength, leading to presumably very weak layers which can be destabilized and jeopardize the stability of the volcano.

The wide range of strength in volcanic rocks is commonly attributed to varying compositions and microstructures inherent in volcanic rock (e.g., Byerlee, 1978; Heap et al., 2014b). Strength of materials has long been described as a function of their porosity (see Paterson and Wong, 2005). In volcanic rock, the porosity is made up of vesicles and microcracks and as such, the relationship presented between strength and permeability, though not commonly investigated, deserves consideration. Early theoretical efforts to model the mechanical properties of porous material have proposed models describing the process of failure for pore-bearing rocks, including the pore-emanated crack model (Sammis and Ashby, 1986), as well as the process of failure of crack-bearing rocks, including the wing-crack model (Ashby

TABLE 3 Summary of all results, including porosity, uniaxial comp	essive strength and Young's Modulus for each sample type by experiment I.D.
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Experiment I.D.	Sample	Test	Strain rate (s ⁻¹)	Temperature	Open porosity	Failure	Young's modulus at
				(°C)	(%)	stress (MPa)	25% UCS (GPa)
	PAC 2013 1	Boom temperature	10-1	20	2.64	173.61	16.84
b	PAC 2013 1	Room temperature	10 ⁻³	20	2 75	162 52	15.80
° C	PAC 2013 1	Room temperature	10 ⁻⁵	20	2.62	111.63	11.04
d	PAC 2013 1	Thermally stressed	10^{-1}	20	2.70	166.95	16.14
e	PAC 2013 1	Thermally stressed	10-3	20	2.87	149.51	14 22
f	PAC 2013 1	Thermally stressed	10 ⁻⁵	20	2.66	115.74	12.44
a	PAC 2013 1	Cvclic	10 ⁻⁵	20	2.44	101.46	13.35–17.80
h	PAC_2013_1	High temperature	10 ⁻¹	935	1.93	167.33	7.87
a	PAC RF 2	Room temperature	10 ⁻¹	20	2.94	173.91	17.48
b	PAC_RF_2	Room temperature	10 ⁻³	20	2.46	104.55	13.33
С	PAC RF 2	Room temperature	10 ⁻⁵	20	2.51	111.26	12.54
d	PAC RF 2	Thermally stressed	10 ⁻¹	20	2.54	171.08	17.87
е	PAC RF 2	Thermally stressed	10 ⁻³	20	3.12	120.03	13.00
f	PAC RF 2	Thermally stressed	10 ⁻⁵	20	2.70	116.69	14.22
g	PAC_RF_2	Cyclic	10 ⁻⁵	20	2.39	135.63	16.22-22.40
a	PAC SCARP	Room temperature	10 ⁻¹	20	6.16	147.38	22.43
b	PAC SCARP	Room temperature	10 ⁻³	20	6.26	158.68	21.64
С	PAC SCARP	Room temperature	10 ⁻⁵	20	6.08	123.18	24.69
d	PAC SCARP	Thermally stressed	10 ⁻¹	20	5.84	143.10	7.14
е	PAC SCARP	Thermally stressed	10 ⁻³	20	6.03	145.58	16.12
f	PAC SCARP	Thermally stressed	10 ⁻⁵	20	5.60	148.87	16.18
q	PAC SCARP	Cyclic	10 ⁻⁵	20	6.03	108.12	17.26-22.66
h	PAC SCARP	High temperature	10 ⁻¹	935	5.41	161.75	8.21
i	PAC_SCARP	High temperature	10 ⁻⁵	935	6.62	125.60	7.18
a	PAC_2010_2	Room temperature	10 ⁻¹	20	26.30	46.01	4.99
b	PAC_2010_2	Room temperature	10 ⁻³	20	10.70	106.51	15.41
С	PAC_2010_2	Room temperature	10 ⁻⁵	20	13.67	45.99	7.66
d	PAC_2010_2	Thermally stressed	10 ⁻¹	20	17.11	49.66	8.53
е	PAC_2010_2	Thermally stressed	10 ⁻³	20	14.50	66.49	10.22
f	PAC_2010_2	Thermally stressed	10 ⁻⁵	20	21.23	22.75	6.00
g	PAC_2010_2	Cyclic	10 ⁻⁵	20	15.34	42.76	8.39-10.97
h	PAC_2010_2	High temperature	10 ⁻¹	935	19.35	59.29	4.26
i	PAC_2010_2	High temperature	10 ⁻⁵	935	15.88	48.77	3.87
а	PAC_RF_1	Room temperature	10 ⁻¹	20	18.17	90.96	17.52
b	PAC_RF_1	Room temperature	10 ⁻³	20	17.03	76.48	19.02
с	PAC_RF_1	Room temperature	10 ⁻⁵	20	16.41	49.92	11.11
d	PAC_RF_1	Thermally stressed	10 ⁻¹	20	18.34	84.00	13.78
е	PAC_RF_1	Thermally stressed	10 ⁻³	20	16.70	72.72	11.95
f	PAC_RF_1	Thermally stressed	10 ⁻⁵	20	17.84	51.15	10.04
g	PAC_RF_1	Cyclic	10 ⁻⁵	20	17.37	66.27	14.51-20.07
h	PAC_RF_1	High temperature	10 ⁻¹	935	18.62	93.22	8.91
i	PAC_RF_1	High temperature	10 ⁻⁵	935	18.96	44.07	4.59
а	PAC_2013_2	Room temperature	10 ⁻¹	20	20.35	64.73	8.78
b	PAC_2013_2	Room temperature	10 ⁻³	20	22.74	38.21	5.92
с	PAC_2013_2	Room temperature	10 ⁻⁵	20	20.17	41.92	7.23
а	PAC_2010_1	Room temperature	10 ⁻¹	20	22.73	63.78	7.91
b	PAC_2010_1	Room temperature	10 ⁻³	20	22.84	63.45	12.51

(Continued)

TABLE 3 C	Continued
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Experiment I.D.	Sample	Test	Strain rate (s ⁻¹)	Temperature (°C)	Open porosity (%)	Failure stress (MPa)	Young's modulus at 25% UCS (GPa)
С	PAC_2010_1	Room temperature	10 ⁻⁵	20	23.34	49.60	11.82
d	PAC_2010_1	Thermally stressed	10 ⁻¹	20	21.20	91.76	13.95
е	PAC_2010_1	Thermally stressed	10 ⁻³	20	20.15	73.95	12.98
f	PAC_2010_1	Thermally stressed	10 ⁻⁵	20	23.01	51.49	9.93
g	PAC_2010_1	Cyclic	10 ⁻⁵	20	20.96	40.17	8.81–11.31
h	PAC_2010_1	High temperature	10 ⁻¹	935	22.53	75.23	8.16
i	PAC_2010_1	High temperature	10 ⁻⁵	935	20.44	63.62	5.63
а	PAC_1974	Room temperature	10 ⁻¹	20	29.90	17.93	3.44
b	PAC_1974	Room temperature	10 ⁻³	20	27.30	27.15	4.52
С	PAC_1974	Room temperature	10 ⁻⁵	20	28.94	15.58	2.90
d	PAC_1974	Thermally stressed	10 ⁻¹	20	29.19	17.06	2.69
е	PAC_1974	Thermally stressed	10 ⁻³	20	28.94	19.73	3.52
f	PAC_1974	Thermally stressed	10 ⁻⁵	20	31.84	12.11	2.63
g	PAC_1974	Cyclic	10 ⁻⁵	20	26.71	15.96	1.99-6.22
h	PAC_1974	High temperature	10 ⁻¹	935	31.91	27.96	4.99
i	PAC_1974	High temperature	10 ⁻⁵	935	30.84	15.67	2.53



and Sammis, 1990). A recent micromechanical study of volcanic rock behavior suggested that the abundance of both vesicles and cracks in volcanic rocks thwart our ability to predict failure using these models (e.g., Heap et al., 2014b). Pores and cracks are fundamental to volcanic systems; the importance of permeability and its influence on gas release vs. build-up in a volcanic systems play an important role on the explosivity of an eruption (e.g., Mueller et al., 2005) but also structural stability (Thomas et al., 2004). Here, we suggest that the strength of rock and the character of the porous and permeable network are intrinsically related (**Figure 4B**) and influence the mechanical properties of volcanic edifices (**Figures 6B,C**).



Strain-rate strengthening is observed and may be a result of increased elasticity as well as crack coalescence lagging behind axial deformation at faster rates (e.g., Jaeger and Cook, 1976). Strain-rate strengthening is observed across all conditions similar to previous studies on volcanic rocks (e.g., Rocchi et al., 2004), but is more effective at high temperature, as was suggested by Lindholm et al. (1974). The strain-rate effect implies that slow strain events, such as downslope movement, creep, or shallow magma pooling, could cause more pervasive damage and be more detrimental to the stability of an edifice than fast strain events such as dike intrusions or ground motion produced during earthquakes.

High temperature strengthening occurred across the sample set. While in some cases it is possible that chemical alteration, partial melting or viscous relaxation of interstitial glass may heal pre-existing fractures, we suggest that under the conditions investigated here, it is more plausible that increased strength relates to expansion and densification (via closure of cracks) and higher elasticity in molecular bonds (see Young's Modulus values, **Table 3**). For example, the experiment temperature (at 935°C) is above the order-disorder transition of plagioclase, which is known to increase seismic velocities (Kono et al., 2008) and may correlate to increased elasticity. Indeed, here we show that rock deformation in a fully brittle regime at high temperatures experience significantly more strain to failure than at room temperature. This characteristic is not unique to the basalts at high temperature, though many other rock types often become weaker with temperature, especially if they undergo alteration (e.g., decarbonation, dehydroxylization; Heap et al., 2012, 2013a,b) or significant thermal expansion (e.g., due to anisotropic expansion in crystals; Meredith and Atkinson, 1982; Zhang et al., 2009).

Experiments conducted at high temperatures cause rocks to be stronger and more elastic, suggesting that areas near magma intrusions, or areas heated over prolonged periods of time, could experience more deformation before failure than previously inferred from knowledge of rock behavior at low temperatures. According to Ebmeier et al. (2013), a lack of surface deformation before eruptions at Pacaya suggests that magma rises directly from the base of the crust without a period of crustal storage. The high temperature effects may thus explain the trivial amount of pre-eruptive ground deformation associated with dike propagation and magma transport in cold rocks prior to eruption at Pacaya. When also considering the strain-rate strengthening effect, these relatively fast dike injection events could also lead to an increase in edifice elasticity, resulting in less pervasive damage and thus less detriment to the edifice forming rock. However, we must note that the results from cyclic experiments show



temperature (insert).

typically deformation at a higher rate of 10⁻¹ s⁻¹ results in a higher apparent Young's Modulus. Noticeably, the material response

not (significantly) affect volcanic edifice stability (Vinciguerra

that if dikes are continually intruded, the country rock could be mechanically weakened over time through strain accumulation, particularly in areas with high elastic modulus values.

Thermally stressing rocks has a complex effect on strength: some cores were slightly weakened, while others were slightly strengthened post-heat treatment. The variable effect of thermal stressing at similar temperatures has also been noted by changes in crack content in other rocks such as andesite (Heap et al., 2014a) and granite (Wang et al., 2013). Although heating and cooling can both induce cracking, there seems to be no systematic response as a function of starting porosity. Volcanic rocks form by cooling and as such, their microstructures and importantly here, microcracks, preserve a record of the cooling history. In our study, the cores were heated and cooled at a rate likely lower than that experienced during cooling upon eruption; it is possible that such mild thermal conditions would minimize the extent of crack damage overprint. According to Heap et al. (2009), so long as the mineralogical assemblage remains intact, microcracks (and subsequent weakening) induced by thermal stressing are likely a function of the original thermal stressing imposed by natural cooling; that is, thermal stressing must be applied at a rate that exceeds natural cooling rate to induce significant damage. Recent studies assessing the effect of fractures induced by thermal stressing concluded that its occurrence would likely

et al., 2005; Heap et al., 2009; Kendrick et al., 2013b). However, we point out that additional mechanical stresses resulting from magmatic forcing during transport would create a zone of damage, increasing the overall heterogeneity of the edifice.

Cyclic loading tests had a variable effect on failure stress and strain, however, they do suggest that repeated dike intrusion or inflation and deflation has the potential to cause edifice weakening by imparting a permanent strain on rocks. The stressstrain curves show hysteresis, and during unloading to 5% of the peak failure stress, the strain remains above zero and gradually increases from cycle to cycle (Figure 8). Such behavior has been observed in other volcanic rocks, including andesite (Kendrick et al., 2013a) and basalt (Heap et al., 2009, 2010), along with sedimentary rocks (e.g., sandstone; Song et al., 2013). Our work further suggests that material with porosities above 5 vol. % can typically absorb stress with fewer permanent damage accumulation (Figure 5B). This elastic response may result from compressibility of the rocks into the porous space, attempting to shut the cracks and vesicles. Elastic response may also be due to changes in porosity isolation as pores are connected through microcracking, with the resultant higher connected porosity allowing for more fluid stress compensation between pores, simultaneously preventing stiffening. This effect would be more abundant in rocks with higher porosity. According to Gurevich (2003), approximately 10% porosity or greater provides enough space for fluid to flow into pores and escape fracture. The implications of cyclic loading/ unloading suggest that volcanic rocks may not behave entirely elastically and accumulate damage. Once imparted, damage in turn affects the resultant material response to stress (changes in strength and Young's modulus), which may favor stress rotation (e.g., Faulkner et al., 2006), with potentially more widespread consequences on the distribution of stress in the edifice.

Conclusions

An experimental investigation of the mechanical properties of basalts at Pacaya volcano has been undertaken to increase our knowledge of contributing variables on slope stability in volcanic edifices. The rocks forming Pacaya volcano are heterogeneous at a range of scales: at the edifice scale, we find the main lithologies are locally heterogeneous due to the limited spread of lava flows as well as to flow emplacement inside earlier collapse scars; at the rock scale, we find that the rocks are macroscopically and microscopically heterogeneous, showing little variation in mineralogy, but strong variation in textures, porosities (formed of pores and cracks) and permeability. The permeability is constrained to increase non-linearly with

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porosity. Laboratory testing of the mechanical behavior shows that the strength decreases with porosity, increases with strain rate, increases with ambient (deformation) temperature, and is unsystematically varied by both thermal stressing, and due to prolonged stressing histories where cycles of loading/ unloading can induce mechanical hysteresis. We anticipate that integration of such process-specific rock properties in modeling of volcanic events will improve hazard analysis interpretation at active volcanoes such as Pacaya.

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Supplementary Material

The Supplementary Material for this article can be found online at: http://journal.frontiersin.org/article/10.3389/feart. 2015.00029/abstract

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Conflict of Interest Statement: The authors declare that the research was conducted in the absence of any commercial or financial relationships that could be construed as a potential conflict of interest.

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