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# A novel method for prediction of extreme wind speeds across parts of Southern Norway

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The paper describes a novel structural reliability method, particularly suitable for multi-dimensional environmental systems, either measured or numerically simulated over a sufficient period, resulting in sufficiently long ergodic time series. This study illustrates the efficiency of the proposed methodology by applying it to predict extreme wind speeds of a group of selected measured sites in Southern Norway in the region near the Landvik wind station. It is well known that wind speeds at different locations are highly non-linear, multidimensional and cross-correlated dynamic environmental responses, which can be challenging to analyse accurately. Unlike other environmental reliability methods, the new method does not require restarting the simulation each time the system fails, e.g., in the case of numerical simulation. In the case of measured environmental system response, an accurate prediction of system failure probability is also possible, as illustrated in this study. Moreover, in contrast to classical reliability methods, the proposed method can handle systems with high dimensionality and cross-correlation between the different dimensions.

#### KEYWORDS

reliability, failure probability, environmental system, wind speeds, Southern Norway

# Highlights

- A novel environmental reliability method has been developed and applied to wind speeds data measured in southern Norway
- Accurate multi-state prediction is performed
- Confidence bands are given

## Introduction

In many practical situations, it would be useful to improve the accuracy of *in situ* wind speeds statistical predictions, as wind speeds are the key part of environmental loads acting on offshore structures and vessels. In this paper, the specific issue of improving extreme wind speed prediction has been addressed. The latter would typically be possible if the location in question has a measurement station more or less in the same area where the recording of wind speed statistics has been going on for several years. In Norway, there

is a grid of such stations located at, e.g., airports and lighthouses. This paper has chosen wind speed time series from a group of selected measured sites in Southern Norway in the region near the Landvik wind station.

It is generally relatively challenging to calculate realistic environmental system reliability using conventional theoretical reliability methods (Madsen et al., 1986; Thoft-Christensen and Murotsu, 1986; Ditlevsen and Madsen, 1996; Melchers, 1999; Choi et at. 2007; Eryilmaz and Kan, 2020; Kan et al., 2020; Zheng et al., 2020). This is usually due to a large number of degrees of system freedom and random variables governing the responses in a real environmental system. In principle, the reliability of a complex environmental system may be accurately estimated straightforwardly either by having enough measurements or by direct Monte Carlo simulations (Naess and Leira and Batsevych, 2009). However, the experimental or computational cost may be unaffordable for many complex dynamic systems in which the long-term statistics of their responses can only be accurately captured using huge data sets. Motivated by the latter argument, the authors have introduced a novel reliability method for environmental systems that uses the average conditional exceedance rate (ACER) method (Naess and Gaidai, 2009; Gaidai et al., 2020; Gao et al., 2020; Sun et al., 2022) for statistical extrapolation to reduce either measurement or computational costs. Extreme value statistics has been a topical matter for some time now, and there are some excellent reviews on the topic, see (Hansen, 2020; Majumdar et al., 2020).

The shortage of previous studies on wind speed predictions is that most of the existing relevant statistical methods are unidimensional (1D) or, at most, bivariate (2D). In the case of multiple measurement locations studied simultaneously, there is a vital need to develop new efficient reliability methods that can tackle cross-correlation between different dimensions (measurements). This study intends to contribute to the latter research challenge.

#### Data and methodology

#### Method

Consider an MDOF (multi-degree of freedom) structure subjected to random ergodic environmental loadings (stationary and homogenous), for example, from the surrounding waves and wind. The other alternative is to view the process as dependent on specific environmental parameters whose variation in time may be modelled as an ergodic process on its own. The MDOF structural response vector process  $\mathbf{R}(t) = (X(t), Y(t), Z(t), \ldots)$  is measured and/or simulated over a sufficiently long time interval (0, T). Unidimensional global maxima over the entire time span (0,T) are denoted as  $X_T \max_{0 \le t \le T} X(t)$ ,  $Y_T \max_{0 \le t \le T} Y(t), Z_T^{\max} = \max_{0 \le t \le T} X(t), \ldots$  By sufficiently long

time T one primarily means a large value of T with respect to the dynamic system auto-correlation time.

Let  $X_1, \ldots, X_{N_X}$  be consequent in time local maxima of the process X(t) at discrete monotonously increasing time instants  $t_1^X < \ldots < t_{N_X}^X$  in (0, T). The analogous definition follows for other MDOF response components  $Y(t), Z(t), \ldots$  with  $Y_1, \ldots, Y_{N_Y}; Z_1, \ldots, Z_{N_Z}$  and so on. For simplicity, all  $\mathbf{R}(t)$  components, and therefore its maxima are assumed to be nonnegative. The aim is to estimate system failure probability, namely probability of exceedance, accurately

$$1 - P = \operatorname{Prob}\left(X_T^{\max} > \eta_X \cup Y_T^{\max} > \eta_Y \cup Z_T^{\max} > \eta_Z \cup \dots\right) \quad (1)$$

with

$$P = \int_{(0, 0, 0, 0, ...)}^{(\eta_X, \eta_Y, \eta_Z, ...)} p_{X_T^{\max}, Y_T^{\max}, Z_T^{\max}, ...} (X_T^{\max}, Y_T^{\max}, Z_T^{\max}, ...)$$
$$dX_T^{\max} dY_{N_Y}^{\max} dZ_{N_z}^{\max} ...$$
(2)

being the probability of non-exceedance for critical values of response components  $\eta_X$ ,  $\eta_Y$ ,  $\eta_Z$ ,...;  $\cup$  denotes logical unity operation «or»; and  $p_{X_T^{\max}, Y_T^{\max}, Z_T^{\max}, ...}$  being joint probability density of the global maxima over the entire time span (0, T).

In practice, however, it is not feasible to estimate the latter joint probability distribution directly  $p_{X_T^{\max}}, y_T^{\max}, Z_T^{\max}, \dots$  due to its high dimensionality and available data set limitations. More specifically, the moment when either X(t) exceeds  $\eta_X$ , or Y(t) exceeds  $\eta_Y$ , or Z(t) exceeds  $\eta_Z$ , and so on, the system is regarded as immediately failed. Fixed failure levels  $\eta_X, \eta_Y, \eta_Z$ ,...are, of course, individual for each unidimensional response component of  $\mathbf{R}(t)$ .  $X_{N_X}^{\max} = \max \{X_j; j = 1, \dots, N_X\} = X_T^{\max}, Y_{N_Y}^{\max} = \max \{Y_j; j = 1, \dots, N_Y\} = Y_T^{\max}, Z_{N_z}^{\max} = \max \{Z_j; j = 1, \dots, N_Z\} = Z_T^{\max}$ , and so on.

the local maxima Next, time instants  $[t_1^X < \ldots < t_{N_x}^X; t_1^Y < \ldots < t_{N_y}^Y; t_1^Z < \ldots < t_{N_z}^Z]$  in monotonously non-decreasing order are sorted into one single merged time vector  $t_1 \leq \ldots \leq t_N$ . Note that  $t_N = \max \{t_{N_X}^X, t_{N_Y}^Y, t_{N_Z}^Z, \ldots\}, N =$  $N_X + N_Y + N_Z + \dots$  In this case  $t_j$  represents local maxima of one of MDOF structural response components either X(t) or Y(t), or Z(t) and so on. That means that having R(t) time record, one just needs continuously and simultaneously screen for unidimensional response component local maxima and record its exceedance of MDOF limit vector ( $\eta_X$ ,  $\eta_Y$ ,  $\eta_Z$ , ...) in any of its components X, Y, Z, .... The local unidimensional response component maxima are merged into one temporal non-decreasing vector  $\vec{R}$  =  $(R_1, R_2, \ldots, R_N)$  in accordance with the merged time vector  $t_1 \leq \ldots \leq t_N$ . That is to say, each local maxima  $R_i$  is the actual encountered local maxima corresponding to either X(t) or Y(t), or Z(t) and so on. See Figure 1 for an illustration of the vector  $\vec{R}$  how-to construction, by means of direct overlapping of two different dimensional processes A and B into the new process C (i.e. vector  $\vec{R}$ ) in real time, containing local maxima of both A and B dimensional processes. Finally, the unified limit vector  $(\eta_1, \ldots, \eta_N)$  is introduced with each component  $\eta_i$  is either  $\eta_X$ ,  $\eta_Y$  or  $\eta_Z$  and



so on, depending on which of X(t) or Y(t), or Z(t) etc., corresponding to the current local maxima with the running index *j*.

Next, a scaling parameter  $0 < \lambda \le 1$  is introduced to artificially simultaneously decrease limit values for all response components, namely the new MDOF limit vector  $(\eta_X^{\lambda}, \eta_Y^{\lambda}, \eta_z^{\lambda}, ...)$  with  $\eta_X^{\lambda} \equiv \lambda \cdot \eta_X$ ,  $\equiv \lambda \cdot \eta_Y$ ,  $\eta_z^{\lambda} \equiv \lambda \cdot \eta_Z$ , ... Is introduced. The unified limit vector  $(\eta_1^{\lambda}, \ldots, \eta_N^{\lambda})$  is introduced with each component  $\eta_j^{\lambda}$  is either  $\eta_X^{\lambda}, \eta_Y^{\lambda}$  or  $\eta_z^{\lambda}$  and so on. The latter automatically defines probability  $P(\lambda)$  as a function of  $\lambda$ , note that  $P \equiv P(1)$  from Eq. 1. Non-exceedance probability  $P(\lambda)$  can be estimated as follows:

$$P(\lambda) = \operatorname{Prob}\left\{R_{N} \leq \eta_{N}^{\lambda}, \dots, R_{1} \leq \eta_{1}^{\lambda}\right\}$$
$$= \operatorname{Prob}\left\{R_{N} \leq \eta_{N}^{\lambda} \mid R_{N-1} \leq \eta_{N-1}^{\lambda}, \dots, R_{1} \leq \eta_{1}^{\lambda}\right\}$$
$$\cdot \operatorname{Prob}\left\{R_{N-1} \leq \eta_{N-1}^{\lambda}, \dots, R_{1} \leq \eta_{1}^{\lambda}\right\}$$
$$= \prod_{j=2}^{N} \operatorname{Prob}\left\{R_{j} \leq \eta_{j}^{\lambda} \mid R_{j-1} \leq \eta_{1j-}^{\lambda}, \dots, R_{1} \leq \eta_{1}^{\lambda}\right\}$$
$$\cdot \operatorname{Prob}\left(R_{1} \leq \eta_{1}^{\lambda}\right)$$
(3)

The following outlines the principle behind a cascade of approximations based on conditioning. The first approximation is a one-step memory approximation and thus resembles a Markov chain approximation. However, it is emphasised that this first approximation is not equivalent to such an approximation (Karpa, 2015). In practice, the dependence between the neighbouring  $R_i$  is not negligible; thus, the

following one-step (will be called conditioning level k = 1) memory approximation is introduced:

$$\operatorname{Prob}\left\{R_{j} \leq \eta_{j}^{\lambda} \mid R_{j-1} \leq \eta_{j-1}^{\lambda}, \dots, R_{1} \leq \eta_{1}^{\lambda}\right\} \approx \operatorname{Prob}\left\{R_{j} \leq \eta_{j}^{\lambda} \mid R_{j-1} \leq \eta_{j-1}^{\lambda}\right\}$$

$$(4)$$

for  $2 \le j \le N$  (will be called conditioning level k = 2). The approximation introduced by Eq. 4 can be further expressed as:

$$\operatorname{Prob}\left\{R_{j} \leq \eta_{j}^{\lambda} \mid R_{j-1} \leq \eta_{j-1}^{\lambda}, \dots, R_{1} \leq \eta_{1}^{\lambda}\right\} \approx \operatorname{Prob}\left\{R_{j} \leq \eta_{j}^{\lambda}\right\}$$
$$R_{j-1} \leq \eta_{j-1}^{\lambda}, R_{j-2} \leq \eta_{j-2}^{\lambda}\right\}$$
(5)

where  $3 \le j \le N$  (will be called conditioning level k = 3), and so on, see (Karpa, 2015). The motivation is to monitor each independent failure that happened locally first in time, thus avoiding cascading local inter-correlated exceedances.

Eq. 5 presents subsequent refinements of the statistical independence assumption. The latter type of approximations captures the statistical dependence effect between neighbouring maxima with increased accuracy. Since the original MDOF process  $\mathbf{R}(t)$  was assumed ergodic and therefore stationary, probability  $p_k(\lambda) \coloneqq \operatorname{Prob}\{R_j > \eta_j^{\lambda} \mid R_{j-1} \le \eta_{j-1}^{\lambda}, R_{j-k+1} \le \eta_{j-k+1}^{\lambda}\}$  for  $j \ge k$  will be independent of j but only dependent on conditioning level k. Thus non-exceedance probability can be approximated as in the Naess-Gaidai method, see (Naess and Gaidai, 2009; Gaidai et al., 2018; Gaidai et al., 2020; Gao et al., 2020) where:

$$P_k(\lambda) \approx \exp\left(-N \cdot p_k(\lambda)\right), \ k \ge 1$$
 (6)

Note that Eq. 6 follows from Eq. 1 by neglectingProb  $(R_1 \le \eta_1^{\lambda}) \approx 1$ , as the design failure probability is usually very small.

Note that Eq. 5 is similar to the well-known mean upcrossing rate equation for the probability of exceedance (Gaidai et al., 2020; Gao et al., 2020; Naess and Gaidai, 2009). There is evident convergence with respect to the conditioning parameterk:

$$P = \lim_{k \longrightarrow \infty} P_k(1); \quad p(\lambda) = \lim_{k \longrightarrow \infty} p_k(\lambda)$$
(7)

Note that Eq. 6 for k = 1 turns into the well-known nonexceedance probability relationship with the mean up-crossing rate function

$$P(\lambda) \approx \exp(-\nu^{+}(\lambda)T); \quad \nu^{+}(\lambda) = \int_{0}^{\infty} \zeta p_{R\dot{R}}(\lambda,\zeta) d\zeta \quad (8)$$

where  $\nu^+(\lambda)$  denotes the mean up-crossing rate of the response level  $\lambda$  for the above assembled non-dimensional vector R(t)assembled from scaled MDOF system response  $(\frac{X}{\eta_X}, \frac{Y}{\eta_Y}, \frac{Z}{\eta_Z}, \dots)$ .

Rice's formula gives the mean up-crossing rate in Eq. 8 with  $p_{R\dot{R}}$  being joint probability density for  $(R, \dot{R})$  with  $\dot{R}$  being time derivative R'(t), see (Rice, 1944). Equation 8 relies on the Poisson assumption, which is that up-crossing events of high  $\lambda$ levels (in this paper, it is  $\lambda \ge 1$ ) can be assumed to be independent. The latter may not be the case for narrow band responses and higher level dynamical systems that exhibit cascading failures in different dimensions, subsequent in time, caused by intrinsic inter-dependency between extreme events, manifesting itself in the appearance of highly correlated local maxima clusters within the assembled vector  $\vec{R} = (R_1, R_2, \ldots, R_N)$ .

In the above, the stationarity assumption has been used. The proposed methodology can also treat the non-stationary case. An illustration of how the methodology can be used to treat non-stationary cases is provided as follows. Consider a scattered diagram of m = 1, ..., M sea states, each short-term sea state having a probability  $q_m$ , so that  $\sum_{m=1}^{M} q_m = 1$ . The corresponding long-term equation is then:

$$p_k(\lambda) \equiv \sum_{m=1}^M p_k(\lambda, m) q_m \tag{9}$$

with  $p_k(\lambda, m)$  being the same function as in Eq. 7 but corresponding to a specific short-term sea state with the number m.

#### Model introduction

The Naess-Gaidai extrapolation model (Naess and Moan, 2013; Gaidai et al., 2022; Xing et al., 2022; Xu et al., 2022) is briefly introduced as it will be used as a basis for the failure

probability distribution tail extrapolation. The method assumes that the class of parametric functions needed for extrapolation in the general case can be modelled similarly to the relation between the Gumbel distribution and the general extreme value (GEV) distribution. The above introduced  $p_k(\lambda)$  as functions are often regular in the tail, specifically for values of $\lambda$  approaching and exceeding1. More precisely, for  $\lambda \ge \lambda_0$ . The distribution tail behaves similar to  $\exp\{-(a\lambda + b)^c + d\}$  with *a*, *b*, *c*, *d* being suitably fitted constants for suitable tail cut-on  $\lambda_0$  value. Therefore, one can write;

$$p_k(\lambda) \approx \exp\{-(a_k\lambda + b_k)^{c_k} + d_k\}, \quad \lambda \ge \lambda_0$$
 (10)

Next, by plotting  $\ln\{\ln(p_k(\lambda)) - d_k\}$  versus  $\ln(a_k\lambda + b_k)$ , often a nearly perfectly linear tail behaviour is observed. It is useful to do the optimisation on the logarithmic level by minimising the following error function *F* with respect to the four parameters*a*<sub>k</sub>, *b*<sub>k</sub>, *c*<sub>k</sub>, *p*<sub>k</sub>, *q*<sub>k</sub>:

$$F(a_k, b_k, c_k, p_k, q_k) = \int_{\lambda_0}^{\lambda_1} \omega(\lambda) \{ \ln(p_k(\lambda)) - d_k + (a_k \lambda + b_k)^{c_k} \}^2 d\lambda,$$
  

$$\lambda \ge \lambda_0$$
(11)

with $\lambda_1$  being a suitable distribution tail cut-off value, namely the largest response value, where the confidence interval width is still acceptable. Optimal values of the parameters  $a_k$ ,  $b_k$ ,  $c_k$ ,  $p_k$ ,  $q_k$  may also be determined using a sequential quadratic programming (SQP) method incorporated in the NAG Numerical Library (Numerical Algorithms Group, 2010).

The weight function  $\omega$  can be defined as  $\omega(\lambda) = \{\ln \operatorname{CI}^+(\lambda) - \ln \operatorname{CI}^-(\lambda)\}^{-2}$  with  $(\operatorname{CI}^-(\lambda), \operatorname{CI}^+(\lambda))$  being a confidence interval (CI), empirically estimated from the simulated or measured dataset, see (Gaidai et al., 2020; Gao et al., 2020; Naess and Gaidai, 2009). When the parameter  $c = \lim_{k \to \infty} c_k$  is equal or close to 1, the distribution approaches to

 $k \rightarrow \infty$  the Gumbel distribution.

For any general ergodic process, the sequence of conditional exceedances over a threshold  $\lambda$  can be assumed to constitute a Poisson process. However, in general, a non-homogeneous one. Thus, for levels of  $\lambda$  approaching 1, the approximate limits of a *p*-% confidence interval (CI) of  $p_k(\lambda)$ can be given as follows:

$$\operatorname{CI}^{\pm}(\lambda) = p_k(\lambda) \left( 1 \pm \frac{f(p)}{\sqrt{(N-k+1)p_k(\lambda)}} \right).$$
(12)

with f(p) being estimated from the inverse normal distribution, for example, f(90%) = 1.65, f(95%) = 1.96. With N being the total number of local maxima assembled in the analysed vector  $\vec{R}$ .

Note that the authors have successfully verified the accuracy of the Naess-Gaidai extrapolation model in previous years for a large variety of one-dimensional dynamic systems (Gaidai et al., 2020; Gao et al., 2020; Naess and Gaidai, 2009; Sun et al., 2022).





#### Data introduction

This section intends to illustrate the efficiency of the abovedescribed methodology, utilising the method to predict the extreme wind speeds at a group of selected measured sites in Southern Norway near the Landvik wind station. The five wind measurement locations are Landvik, Kjevik, Lindesnes Fyr, Lista Fyr and Oksøy Fyr. Their measured daily highest daily wind cast speeds are defined as five environmental system components (dimensions)  $X, Y, Z, \ldots$ , thus constituting an example of a five-dimensional (5D) environmental system. The definition of design values is problem-specific and has to be appropriately considered by the end-user. As an example for this paper, the unidimensional design value for each dimension, i.e., the value in which the wind speed is unacceptable at each location, is chosen as twice the maximum daily highest daily wind cast speed during the observation period 2010-2020 for each of the five selected wind measurement locations.

Figure 2 presents wind measurement locations according to the Norwegian Meteorological Institute, 2021. The blue circle indicates the area of interest. The daily largest wind cast speed at the Landvik location during the years 2010–2020 is shown in Figure 3.

$$X \to \frac{X}{\eta_X}, \quad Y \to \frac{Y}{\eta_Y}, \ Z \to \frac{Z}{\eta_Z}, \dots$$
 (13)





making all five responses non-dimensional and having the same failure limit equal to 1.

Next, all the local maxima from five measured time series are merged into one single time series by keeping them in time nondecreasing order:  $\vec{R} = (\{X_1, Y_1, Z_1, \ldots\}, \ldots, \{X_N, Y_N, Z_N, \ldots\})$  with each set  $\{X_j, Y_j, Z_j, \ldots\}$  being sorted according to nondecreasing times of occurrence of these local maxima. Figure 4 presents an example of the non-dimensional assembled vector  $\vec{R}$ , consisting of assembled local maxima of raw daily largest wind cast speed data. The failure probability distribution tail extrapolation was performed towards 100 years return period. Note that vector  $\vec{R}$  does not have physical meaning on its own, as it is assembled of completely different response components. The index *j* is just

a running index of local maxima encountered in a nondecreasing time sequence.

Figure 5 presents extrapolation according to Eq. 9 towards failure state with 25 years return period, which is 1, and somewhat beyond,  $\lambda = 0.15$  cut-on value was used. The dotted lines indicate extrapolated 95% confidence interval according to Eq. 10. According to Eq. 6,  $p(\lambda)$  is directly related to the target failure probability 1 - P from Eq. 1. Therefore, in agreement with Eq. 6, the system failure probability,  $1 - P \approx 1 - P_k(1)$  can be estimated. Note that in Eq. 5, N corresponds to the total number of local maxima in the unified response vector  $\vec{R}$ . The conditioning parameter k = 4 was found to be sufficient due to occurrence of convergence with respect to k; see Eq. 6. Figure 5 exhibits a relatively narrow 95% CI; the latter is due to a substantial amount of data used in this study, namely the recent 10 years of continuously measured data.

As demonstrated, while being novel, the above-described methodology has a clear advantage in efficiently utilising the available measured data set. This is due to its ability to treat system multi-dimensionality, as well as being able to perform accurate extrapolation based on a relatively limited data set.

## Conclusion

Classic reliability methods dealing with time series do not have the advantage of dealing efficiently with systems possessing high dimensionality and cross-correlation between different system responses. The key advantage of the introduced methodology is its ability to study the reliability of highdimensional non-linear dynamic systems.

This paper studied the Norwegian highest largest wind cast speeds data set, in the region near Landvik wind station, observed during the recent decade 2010-2020. A novel environmental reliability method was applied to predict the occurrence of extreme winds in the area of interest within the time horizon of the next 100 years. It is shown that the proposed method produced a reasonable confidence interval. Thus, the suggested methodology may become an appropriate tool for various non-linear dynamic systems reliability studies. Both measured and numerically simulated time series responses can be analysed. Further, unlike other reliability methods, the new method does not require restarting numerical simulation each time it fails, for example, in the case of Monte Carlo

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simulations. Finally, the suggested methodology can be used in a wide range of engineering areas of applications. [Andersen and Jensen, 2014, Ellermann, 2008, Falzarano et al., 2012, Su, 2012.

#### Data availability statement

The original contributions presented in the study are included in the article/Supplementary Material, further inquiries can be directed to the corresponding author.

## Author contributions

All authors listed have made a substantial, direct, and intellectual contribution to the work and approved it for publication.

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## Conflict of interest

The authors declare that the research was conducted in the absence of any commercial or financial relationships that could be construed as a potential conflict of interest.

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