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Study on fatigue characteristics of anti-stripping sandstone asphalt mixture after water immersion

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The insufficient adhesion between sandstone aggregates and asphalt binders makes the mixture susceptible to fatigue cracking under moisture conditions, limiting the widespread application of sandstone aggregates. Thus, two non-amine anti-stripping agents, XT-1 (XT) and PM-JL-06A (PM), were selected to enhance the adhesion properties of sandstone asphalt mixtures. First, the type and optimal dosage of anti-stripping agents were determined through three indicators and the boiled method test. Then, four point bending strength tests with different loading rates were carried out on the immersion and non-immersion sandstone asphalt mixture, and on this basis, four point bending fatigue test and fatigue residual strength test were carried out. The results show that compared to the XT anti-stripping agent, the PM agent more effectively enhances the adhesion of sandstone–asphalt mixtures, raising the adhesion grade from level 3 to above level 4 at an optimal dosage of 0.4%. Compared with conventional SBS-modified sandstone asphalt mixtures, PM-modified mixtures showed a 5.3% average increase in dynamic load strength under dry conditions and an 8.4% improvement under water immersion conditions. Furthermore, the addition of PM anti-stripping agent increased the fatigue life by 33.8% under dry conditions and 38.9% under water immersion conditions. Notably, PM modification significantly reduced the damage factor and damage sensitivity of the mixture under cyclic loading while substantially decreasing the fatigue strength decay rate.

KEYWORDS

sandstone asphalt mixture, anti-stripping agent, adhesion, fatigue residual strength, fatigue damage evolution model

1 Introduction

Asphalt pavements, as a critical component of road infrastructure, dominate high-grade highway construction in China owing to their advantages of low construction costs, reduced driving noise, and enhanced safety and comfort (Jin et al., 2021; Ma et al., 2016). However, with the rapid development of road construction, the scarcity of high-quality aggregates, such as basalt, limestone, and diabase, along with the high procurement costs, became a particularly prominent issue. Therefore, road engineers gradually focused on seeking alternative materials. Currently, many road engineers have studied the feasibility of using

steel slag (Zhao et al., 2024; Lei B. et al., 2025; Lei Z. et al., 2025; Liu et al., 2025), ceramic waste (Sun et al., 2024; Lawanwadeekul, et al., 2025; Meena et al., 2022), blast furnace slag (Mahto and Sinha, 2022; Lei B. et al., 2025; Silva et al., 2023) and sandstone (Zhang et al., 2022; Zaidi et al., 2022; Yang, 2018) as road construction materials, and have achieved a number of re-search findings. Among them, relevant studies (Du et al., 2019; Lei et al., 2024) showed that sandstone aggregates possessed characteristics such as dense texture, high hardness, and strong wear resistance, offering superior mechanical properties compared to the traditionally used limestone and diabase. However, some studies (Fan et al., 2023; Liu and Chen, 2024), showed that sandstone aggregates exhibited defects in adhesion with both matrix asphalt and modified asphalt, resulting in insufficient interfacial bonding strength to meet the technical requirements for surface layers in highways. This, in turn, led to the failure of the mixture's water stability indicators. If directly used in asphalt pavement construction, it would severely affect the service life of the asphalt pavement. Therefore, it is necessary to take appropriate measures to improve the adhesion between sandstone aggregates and asphalt binder, in order to make full use of local materials.

With the swift rise in traffic volume and axle load, asphalt pavements progressively develop issues like cracking, rutting, disintegration, and surface abrasion under repeated traffic loading (Moreno and Rubio, 2013; Zhang et al., 2021; Ling et al., 2021). Among these pavement types, sandstone asphalt pavement is particularly prone to fatigue cracking under repeated wheel loading. Nevertheless, there has been a notable scarcity of systematic investigations into the fatigue properties of sandstone asphalt mixtures subjected to cyclic loading, and correspondingly, few effective enhancement strategies have been put forward. Currently, direct tension (Jiang et al., 2018), uniaxial compression (Mirzaiyanraheh et al., 2022), splitting (Ali et al., 2015), and four-point bending fatigue tests (Izadi et al., 2018) are the conventional methods for studying the damage evolution characteristics of asphalt mixtures. These methods are widely used due to their advantages of short test cycles, simple operation, and low cost. Compared with the other three tests, the four-point bending fatigue test simulates pavement loading stress more realistically, and the fatigue crack location remains fixed, improving the rationality of evaluating the performance of non-uniform materials (Meng et al., 2024).

Research methods for fatigue cracking mainly include the phenomenological method (Fan et al., 2022; Ameri et al., 2017), mechanical approximation method (Li et al., 2022), damage mechanics method (Corradini and Cerni, 2020), and energy method. The damage mechanics method views fatigue as a dynamic process of continuous accumulation and evolution of internal material damage. When damage develops to a critical state and causes material failure, fatigue fracture occurs. This theory can unify the descriptions of fatigue crack initiation and propagation stages and enable a continuous analysis of the entire fatigue failure process (Yu et al., 2012) making it favored by many researchers. The core of this method lies in selecting damage variables with clear physical meaning and ease of experimental measurement to quantify material damage, and in establishing the quantitative relationship between loading action and material performance degradation through damage evolution equations. Studies have shown that the

residual strength of asphalt mixtures during cyclic loading is not only easy to measure but can also directly reflect the performance degradation of pavement materials, demonstrating considerable research potential (Ni et al., 2017). The fatigue residual strength test measures the residual strength of asphalt mixture specimens after different numbers of loading cycles throughout the fatigue life cycle, in order to reflect the strength variation law of asphalt mixture during the fatigue process.

In summary, to enhance the water stability of sandstone asphalt mixtures, PM and XT anti-stripping agents were incorporated into SBS-modified asphalt in this study. The optimal anti-stripping agent type and dosage were determined through three key indices and the boiled method test. Subsequently, four-point bending dynamic strength tests at varying loading rates were conducted on water immersed sandstone asphalt mixtures. Further, four-point bending fatigue tests and fatigue residual strength tests under different stress ratios were performed to comprehensively evaluate the efficacy of anti-stripping agents in improving fatigue performance after water damage.

2 Materials and methods

2.1 Raw material test

The aggregates used in this study are from the sandstone gravels mined in Taozhi notch project section of Cangzhao Expressway in Guangxi. The filler is limestone. Aggregates were collected on-site (Figure 1), and test results are summarized in Tables 1, 2.

According to existing literature, the selection of anti-stripping agents for asphalt binders primarily considers economic viability, construction feasibility, and gain stability (Nazirizad et al., 2015; Arabani et al., 2014). Lime- and fly ash-based anti-stripping agents often suffer from inconsistent dispersion, resulting in non-uniform adhesion within asphalt mixtures. While nanomaterial additives are cost-prohibitive, amine-based agents suffer from poor thermal stability. Furthermore, many novel anti-stripping agents remain at the laboratory research stage and lack engineering validation. Consequently, non-amine anti-stripping agents emerge as the optimal choice. In this study, the non-amine agents XT-1 (XT) and PM-JL-06A (PM), developed by Littel Company, were selected to enhance the adhesion between sandstone aggregates and asphalt binders. The parameters of these anti-stripping agents are summarized in Table 3.

2.2 Preparation of modified asphalt with anti-stripping agent

According to the specification for modified additives of asphalt mixture, *Asphalt mixture modified additives Part IV Anti-stripping agent* JT/T 860.4–2014 (Ministry of Transport of the Peoples Republic of China, 2014), the SBS-modified asphalt is first heated to 160°C in an oven. The anti-stripping agent is then added according to the predetermined dosage. A heating furnace is employed to maintain the mixing temperature at approximately 160°C, and a glass rod is used to thoroughly mix the anti-stripping



TABLE 1 Technical performance parameters of aggregates.

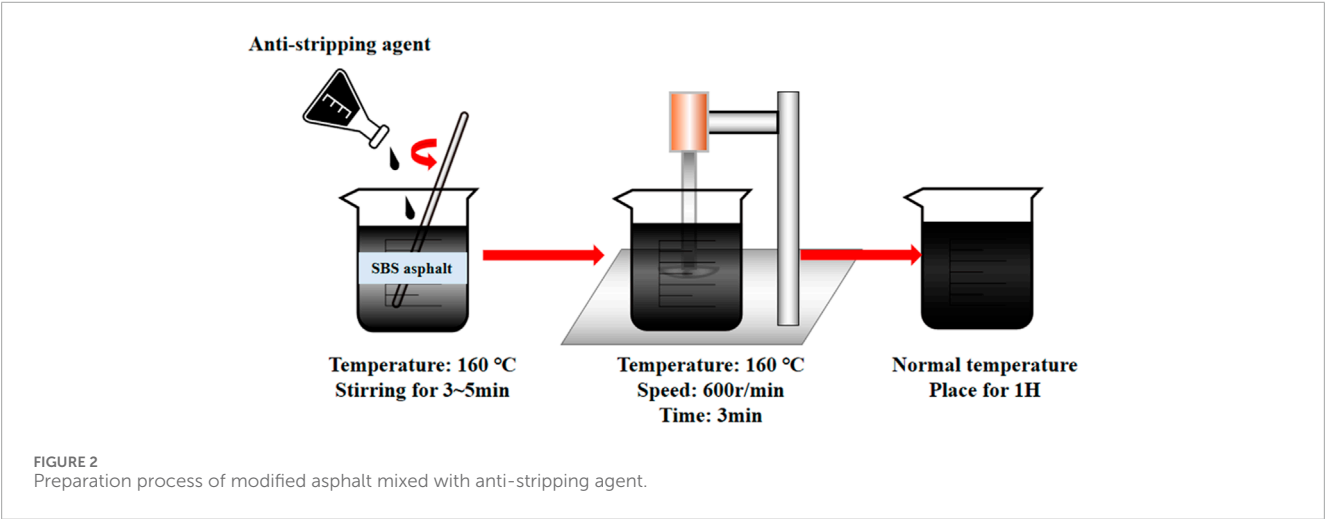
Type	Test index	Measured results	Specification requirements	Test method
Coarse aggregate	Crushing value of stone (%)	15.8	≤28	T 0316
	Los Angeles abrasion value (%)	11.3	≤30	T 0317
	Polished value (%)	47	≥42	T 0321
	Apparent relative density (g/cm ³)	2.73	≥2.5	T 0304
	Water absorption (%)	0.44	≤3.0	T 0304
Fine aggregate	Apparent relative density (g/cm ³)	2.73	≥2.5	T 0328
	Robustness (%)	25	≥12	T 0340
	Silt content (%)	1.6	≤3.0	T 0333
	Sand equivalent (%)	69	≥60	T 0334
	Angularity (s)	33	≥30	T 0345

TABLE 2 Test results of technical indicators of SBS modified asphalt.

Test items	Unit	Detection result	Specification requirements	Test basis
Penetration (25°C, 100g, 5S)	0.1 mm	48.33	30–60	T0604
Softening point (global method)	°C	73.35	≥60	T0606
Kinematic viscosity (135°C)	Pa·s	2.33	≤2.8	T0620
Ductility (5°C, 5 cm/min)	cm	31.61	≥20	T0605

TABLE 3 parameter index of anti-stripping agent.

Project		Index
PM-JL-06A	Ingredients	Non amine organic compound
	Failure temperature (°C)	>300
	Security	Safe, environmentally friendly, non-toxic, odorless and pollution-free
	Solubility	Good compatibility with hot melt asphalt
XT-1	Ingredients	Nonamine active agent
	Failure temperature (°C)	>300
	Security	Safe, environmentally friendly, non-toxic, odorless and pollution-free
	Solubility	Good compatibility with various asphalt



agent with the asphalt, ensuring uniform heating of the mixture. Subsequently, the mixture is subjected to shearing using a shear machine operating at 600 r/min for 3 min. Following this mechanical mixing, the agitator is deactivated and the mixture is maintained at the processing temperature for 1 h to complete the maturation process. The detailed procedural steps are illustrated in Figure 2.

2.3 Grading design and optimal asphalt-aggregate ratio

The AC-20 asphalt mixture gradation, commonly employed in the middle surface layer of expressways, was selected as the research subject, with its gradation curve presented in Figure 3. Using the Marshall test method, the optimal asphalt-to-aggregate ratio for the modified asphalt mixture was determined to be 4.8%. The performance parameters corresponding to this optimal ratio are summarized in Table 4.

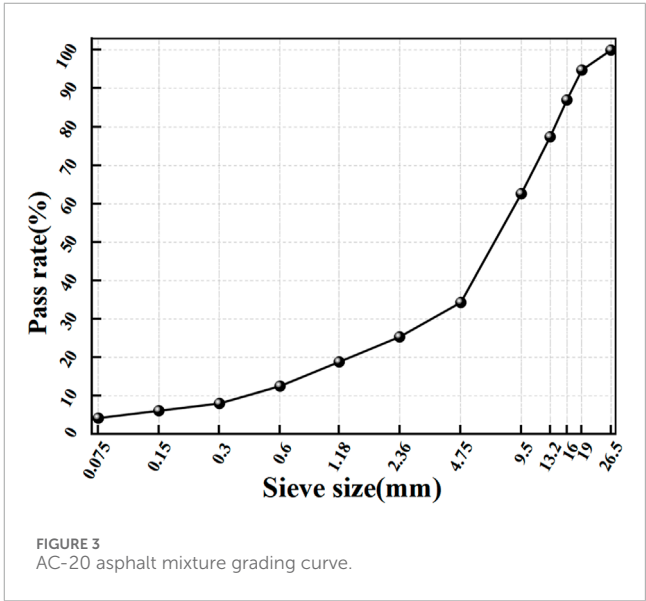


TABLE 4 Marshall test results under the optimal asphalt-aggregate ratio.

Gradation types	Asphalt-aggregate ratio (%)	Relative density of gross volume (g/cm ³)	Voidage VV (%)	Saturation VFA (%)	Stability (KN)	Flow value (0.1 mm)
AC-20	4.8	2.421	5.3	74.2	14.8	3

2.4 Test method

Based on existing literature (Nazirizad et al., 2015; Arabani et al., 2014) and manufacturer-recommended dosage ranges, PM and XT anti-stripping agents (0.1%–0.5% by mass) were blended with SBS-modified asphalt to prepare 10 modified asphalt variants. Following the Standard Test Procedure for Asphalt and Asphalt Mixture of Highway Engineering JTG E20-2011 (Ministry of Transport of the Peoples Republic of China, 2011), conventional tests (penetration, ductility, and softening point) and anti-stripping performance evaluation using the boiling method (T0663-2000) were conducted to determine the optimal anti-stripping agent dosage.

The aggregate used in this study originated from the Taozhi Notch section of the Cangzhao Expressway in Guangxi, China. Given Guangxi's annual temperature range of 10°C–23°C, a test temperature of 15°C was selected for asphalt mixture fatigue analysis. To simulate water damage effects, four-point bending fatigue specimens were conditioned using the water immersion Marshall test method (T 0709–2011). Specimens were water immersed in a constant-temperature water bath at 15°C for 48 h to replicate prolonged environmental water exposure. Subsequent four-point bending dynamic strength tests were performed at seven loading rates: 0.01 MPa/s, 0.02 MPa/s, 0.05 MPa/s, 0.1 MPa/s, 0.2 MPa/s, and 0.5 MPa/s. Using baseline dynamic strengths at 0.01 MPa/s, 0.05 MPa/s, and 0.2 MPa/s, five stress ratios (0.2, 0.3, 0.4, 0.5, and 0.6) were applied to conduct bending fatigue tests and fatigue residual strength evaluations.

When conducting the fatigue residual strength test, it is essential to choose specimens subjected to various cyclic loading times in order to ascertain the residual strength. This step is crucial for elucidating the strength degradation characteristics of asphalt mixtures during the fatigue process. The selection of different cyclical loading times typically adheres to the following principles: 1. To prevent specimen failure before reaching the target duration, the number of cycles should not be excessively high. 2. Adequate intervals between cyclic loading durations must be maintained to ensure the test results comprehensively reflect the strength attenuation pattern. In this study, based on the research findings of Ni et al. (2017) and others, 20%, 50%, 65%, and 80% of fatigue life were selected as the target cyclic loading durations.

2.5 Specimen preparation

The beam specimen forming process is shown in Figure 4. The quantity of asphalt mixture required is determined based on the

specimen size, target void ratio, and maximum theoretical density. After hot mixing, the material is placed into a mold and compacted using a vibrating wheel roller, with the specimen height monitored in real time. Once formed, the slab is allowed to stand at a temperature not exceeding 35°C for 24 h, after which it is cut into four small beam specimens with dimensions of 380 × 63 × 50 mm.

3 Results and discussion

3.1 The optimal dosage of anti-spalling is determined

Through conducting the standard asphalt three index test (ductility, softening point, and penetration) along with the boiling method test, the accurate optimal dosage of XT and PM anti-stripping agents was determined, with the comprehensive test findings visually summarized and presented in Figures 5, 6.

From Figure 5, it can be observed that the addition of the anti-stripping agent has no significant effect on the softening point and penetration of the two asphalt types. However, the ductility shows a trend of first increasing and then decreasing. This phenomenon can be explained by the composition and physical behavior of asphalt. The softening point is primarily influenced by the asphaltene content and intermolecular forces, while penetration is associated with the balance between light and heavy components in the asphalt. Since the anti-stripping agent does not alter the proportions of saturates, aromatics, and asphaltenes, it does not significantly affect the softening point or penetration (Lu et al., 2023). In contrast, the observed changes in ductility can be attributed to the influence of agent on the mechanical properties of asphalt. The anti-stripping agent reduces the elastic recovery capacity and resistance to deformation across a wide frequency domain at high temperatures. Consequently, it enhances the flexibility and ductility of asphalt under low-temperature conditions (Shu et al., 2022). Through the evaluation of adhesion by boiling method, it was found that when the content of two anti-stripping agents exceeded 0.2%, the adhesion grade of asphalt and sandstone reached above grade 4. When the content of PM anti-stripping agent and XT anti strip-ping agent is 0.4% and 0.5%, respectively, the adhesion coefficient of asphalt reaches the peak. Compared with XT anti-stripping agent, PM anti-stripping agent has more significant effect on improving the adhesion of asphalt. In conclusion, PM anti strip-ping agent has better effect on improving the overall performance of asphalt, and further research is carried out with the dosage of 0.4% PM anti-stripping agent.

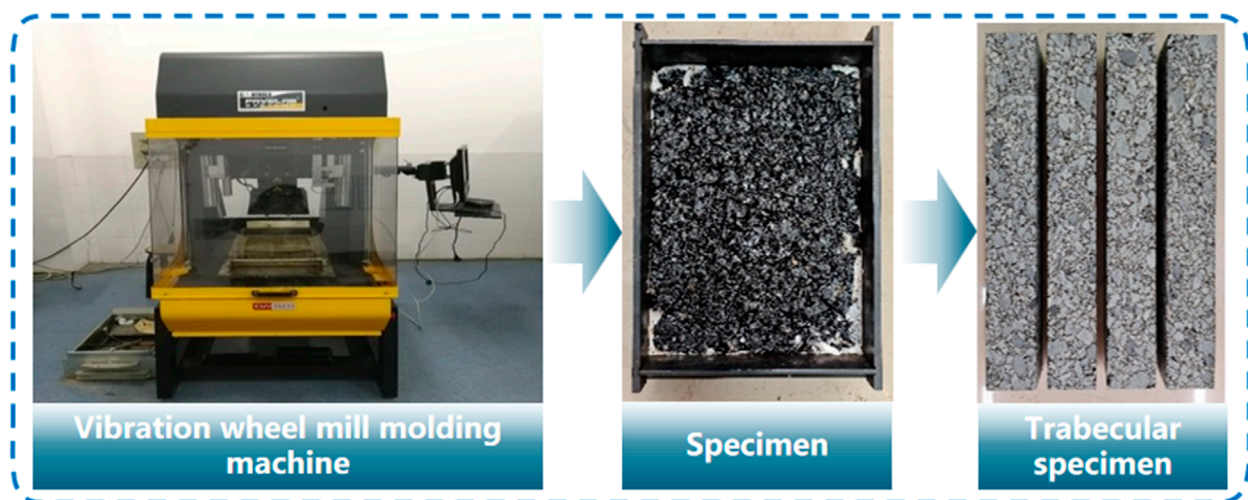


FIGURE 4
Forming process of trabecular specimen.

3.2 Dynamic strength characteristics of sandstone asphalt mixture considering the influence of water immersion and loading rate

Under the condition of 15 °C, carry out four point bending dynamic load strength tests at different loading rates for SBS and sandstone asphalt mixture mixed with PM anti-stripping agent after water immersing the test piece in water for 48 h, and use Equation 1 to fit the variation of bending tensile strength value with the loading rate. The results are shown in Figure 7, and the fitting equation under different conditions is shown in Equations 2–5.

$$R = a \cdot v^b \quad (1)$$

Where, R is the dynamic load strength, MPa; v is the loading rate, MPa/s; a and b are fitting parameters, a reflect the strength of asphalt mixture, b reflect the sensitivity of asphalt mixture to loading rate.

SBS:

$$R_{SBS} = 9.001 \cdot v^{0.225} \quad (2)$$

SBS(immerision):

$$R_{SBS-I} = 8.576 \cdot v^{0.224} \quad (3)$$

SBS + PM:

$$R_{SBS+PM} = 9.201 \cdot v^{0.225} \quad (4)$$

SBS + PM(immerision):

$$R_{SBS+PM-I} = 8.768 \cdot v^{0.224} \quad (5)$$

Where, R_{SBS} is the dynamic load strength of SBS modified asphalt under non immersed conditions, MPa; R_{SBS-I} is the dynamic load strength of SBS modified asphalt under immersion conditions, MPa; R_{SBS+PM} is the dynamic load strength of SBS + PM asphalt under non

immersed conditions, MPa; $R_{SBS+PM-I}$ is the dynamic load strength of SBS + PM asphalt under immersion conditions, MPa;

At 15°C, the dynamic load strength and loading rate of the two kinds of sandstone asphalt mixtures under water immersion and non-water immersion conditions are power functions, and the initial strength growth rate is fast and then slow. Compared with SBS sandstone asphalt mixture, the average dynamic load strength of PM + SBS sandstone asphalt mixture under non-water immersion conditions increased by 5.3%, while the average dynamic load strength under water immersion conditions increased by 8.4%, which indicates that adding PM anti-stripping agent can effectively improve the strength of sandstone, which is mainly due to the improvement of asphalt aggregate interface adhesion and the integrity of the mixture under water immersion conditions.

3.3 Fatigue properties of sandstone asphalt mixture after water immersion

In order to study the effect of adding PM type anti-stripping agent on the fatigue characteristics of sandstone asphalt mixture after water immersion, based on the four point bending strength of 0.01 MPa/s, 0.05 MPa/s and 0.2 MPa/s, the stress ratios of 0.2, 0.3, 0.4, 0.5 and 0.6 were selected to determine the fatigue loading stress, and the four point bending fatigue test of sandstone asphalt mixture at 15°C and loading frequency of 10Hz was carried out, and the test results were fitted by the S-N fatigue equation shown in Equation 6. The results are shown in Figure 8, and the fitting parameters are shown in Table 5.

$$\lg N_f = \lg k - n \cdot \lg t \quad (6)$$

Where, N_f is the fatigue life of the specimen, times; k and n are fitting parameters, which reflect fatigue life and fatigue sensitivity, respectively; t is the stress ratio.

It can be seen from the above that the fatigue life of SBS sandstone asphalt mixture and SBS + PM sandstone asphalt mixture

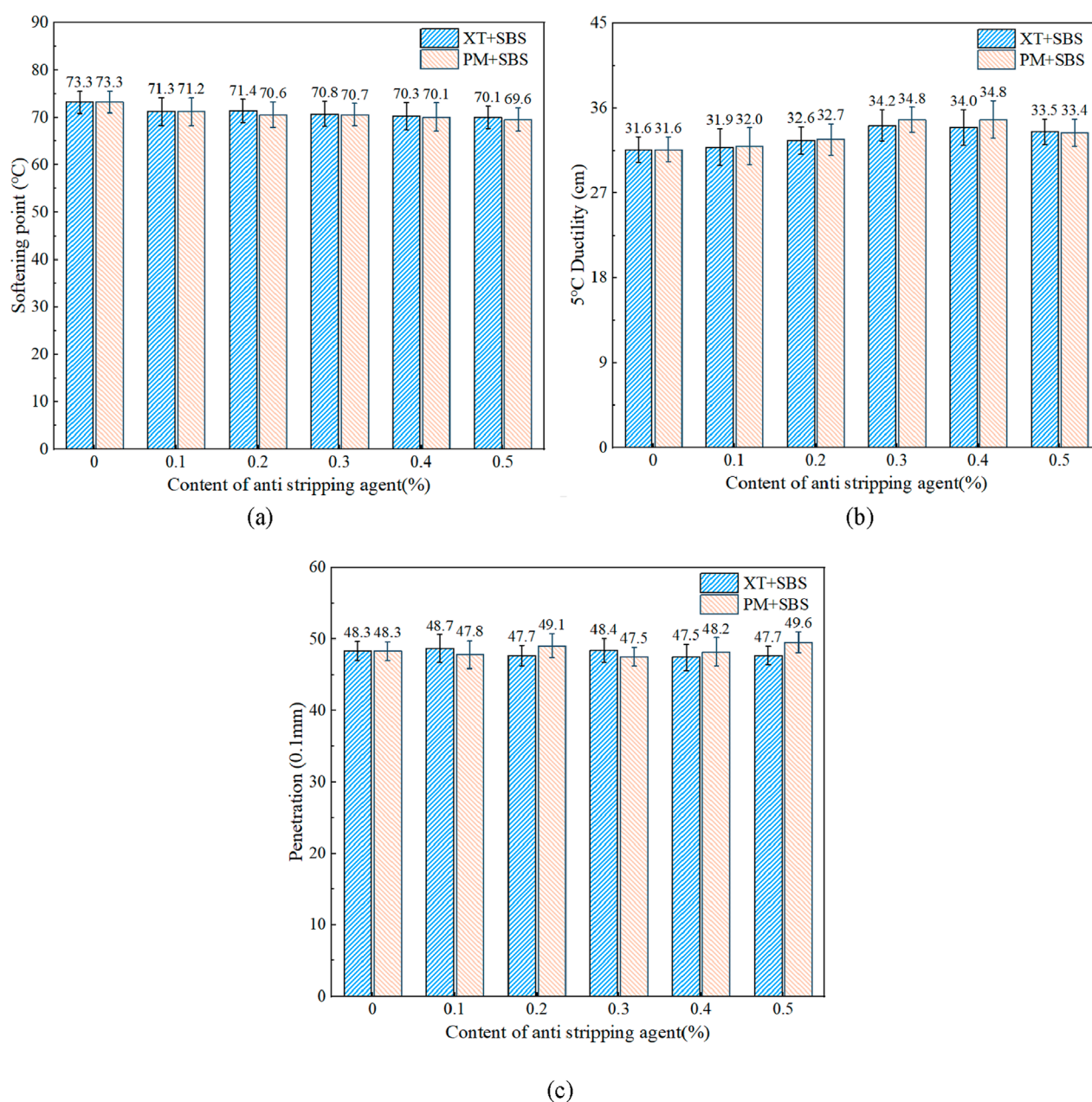
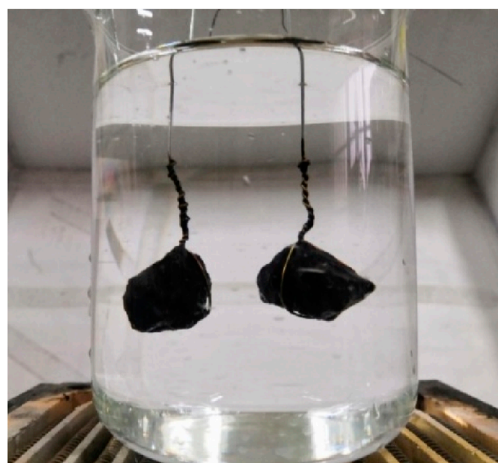


FIGURE 5
Effect of anti-stripping agent content on three indexes of asphalt; (a) Softening; (b) Ductility; (c) Penetration.

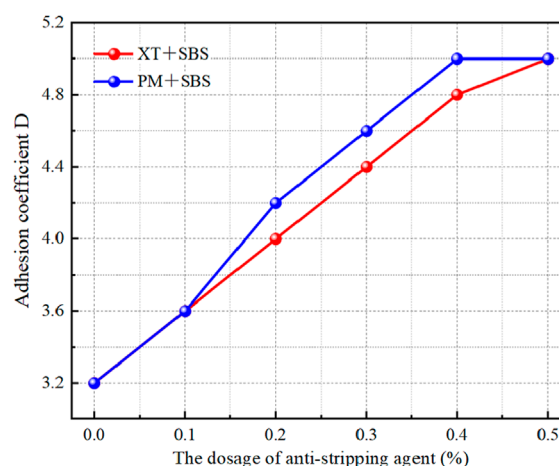
after water immersion test is significantly attenuated compared with that of non-water immersion condition. The fatigue life of the mixture without anti-stripping agent decreased by 25%–48%, while the fatigue life of the mixture with PM anti-stripping agent decreased by 12%–30%. It is worth noting that adding PM type anti-stripping agent can increase the fatigue life of sandstone asphalt mixture under the condition of non-water immersion by 33.8%, and under the condition of water immersion by 38.9%, which indicates that PM type anti-stripping agent can effectively improve the water damage resistance of asphalt mixture.

Parameters k and n in the $S-N$ equation represent fatigue durability and stress sensitivity respectively. It can

be seen from Table 7 that compared with SBS sandstone asphalt mixture, the k value of SBS + PM sandstone asphalt mixture under the condition of no soaking is increased by 22.1%, and the n value is reduced by 19.8%; The k value increased by 25.3% and the n value decreased by 23.1% under the condition of water immersion. This shows that PM anti-stripping agent can effectively improve the water damage resistance of sandstone asphalt mixture, and effectively improve the water stability of the mixture by strengthening the interface bonding between asphalt and sandstone aggregate, so as to maintain higher durability of the material under fatigue load; At the same time, the stress sensitivity is reduced, and finally the synergistic improvement of fatigue resistance and water stability is realized.

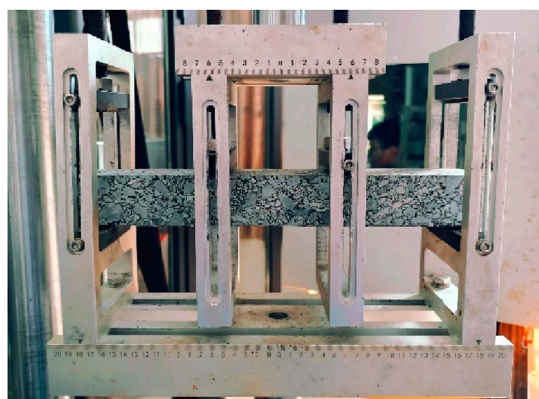


(a)

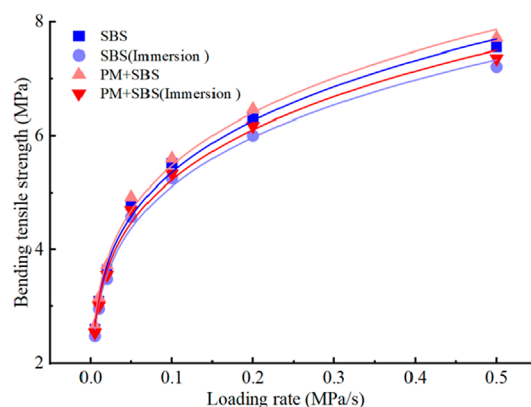


(b)

FIGURE 6 Effect of anti-stripping agent content on asphalt adhesion coefficient; (a) Boiling test; (b) test result.



(a)



(b)

FIGURE 7 Test results of flexural tensile strength of sandstone asphalt mixture at different temperatures; (a) Four point bending test of sandstone asphalt mixture; (b) Fitting curve of flexural tensile strength of sandstone asphalt mixture.

3.4 Study on the degradation pattern of residual strength of sandstone asphalt mixtures under the effects of water immersion and cyclic loading

Considering the insufficient fatigue life of sandstone asphalt mixtures under stress ratios of 0.5 and 0.6 for conducting residual strength characterization, this study implemented four-point bending fatigue residual strength tests based on predetermined damage levels corresponding to cyclic loading stages. The experimental protocol was established through systematic analysis of four-point bending fatigue test results obtained at stress ratios of 0.2, 0.3, and 0.4. Specifically, critical loading cycles corresponding

to 20%, 50%, 65%, and 80% damage progression thresholds were determined through fatigue life prediction modeling. The resultant residual strength evolution patterns under progressive damage conditions are systematically presented in Table 6, demonstrating significant degradation characteristics associated with accumulated fatigue damage.

The experimental findings demonstrate that both SBS-modified and SBS + PM-reinforced sandstone asphalt mixtures exhibit pronounced nonlinear residual strength degradation under water immersed and non-water immersed conditions. Notably, these materials share a consistent three-stage degradation progression (incubation, accelerated deterioration, and terminal failure), as evidenced by comparative analysis of fatigue damage

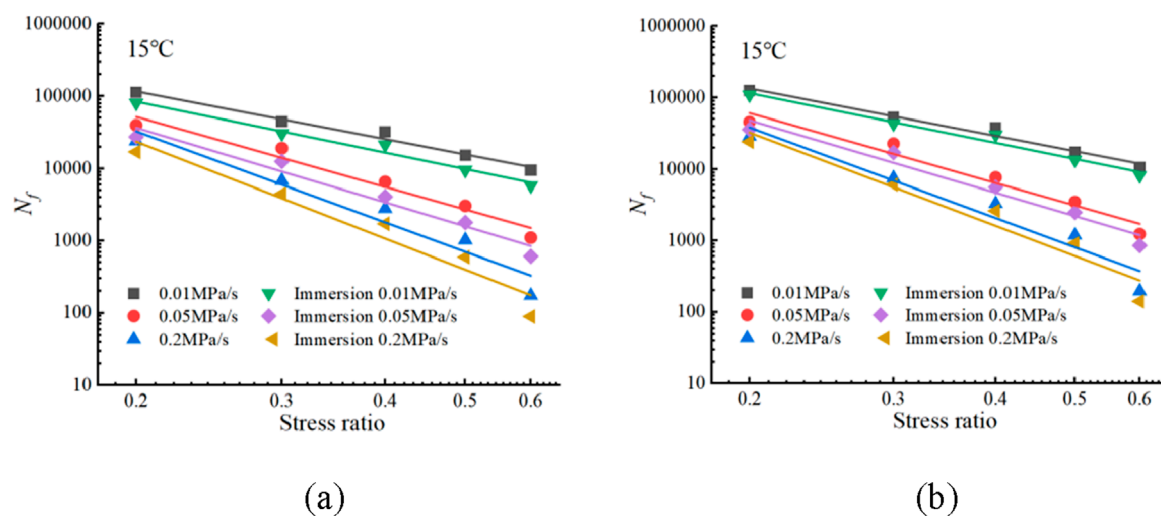


FIGURE 8 Fatigue life fitting curve based on stress ratio; (a)SBS sandstone asphalt mixture; (b)SBS + PM Sandstone asphalt mixture.

TABLE 5 Fitting parameters of fatigue equation.

Type of mixture	loading rate $v(\text{MPa/s})$	k (Immersion)	k	n (Immersion)	n
SBS	0.01	2,831	1,585	2.535	2.372
	0.05	196	98	3.784	3.594
	0.2	20	9	5.113	4.800
PM + SBS	0.01	3,435	4,764	2.122	2.011
	0.05	324	493	2.974	2.861
	0.2	59	84	3.697	3.593

evolution pathways: In the initial damage phase (0%–40% damage degree), the intergranular interlocking effect maintains effective stress transmission capacity, resulting in a gradual decline of residual strength accompanied by stable microcrack propagation within the composite microstructure. During the intermediate degradation stage (40%–80% damage degree), interfacial debonding evolution and interconnected crack network development induce significant degradation in material stiffness, manifesting as accelerated deterioration characteristics of residual strength. Upon reaching the critical failure threshold (>80% damage degree), percolation damage through the aggregate-matrix interface leads to coalescence of macro-cracks, triggering catastrophic strength reduction and abrupt fracture failure accompanied by structural instability (Zhou et al., 2020).

To elucidate the strength degradation mechanism of sandstone asphalt mixtures under cyclic four-point bending loading, a damage index D is defined as the ratio of residual strength $R_{x\%}$ to initial strength R_0 at specified loading cycles. The critical damage threshold is determined when the accumulated loading cycles reach the fatigue life ($N=N_f$), enabling the division of the fatigue process

into two distinct phases: damage accumulation phase and strength failure phase. A fatigue damage evolution equation is subsequently formulated through Equation 7. The evolution curves and fitting parameters are shown in Figure 9 and Table 7, respectively.

$$D(N) = 1 - \frac{R_{x\%}}{R_0} = 1 - \left(1 - \frac{N}{N_f}\right)^u \quad (7)$$

Where, $R_{x\%}$ is the remaining strength value under different damage levels in a four-point bending test; R_0 is the initial intensity value of the four-point bending; u is the model parameter, indicating damage sensitivity.

According to the relationship between the fitting curve parameter u and the stress ratio, the damage evolution models of SBS and SBS + PM sandstone asphalt mixtures under water immersion and non-water immersion conditions were established, as shown in Equations 8–11.

SBS:

$$R_{\text{SBS}-x\%} = R_{\text{SBS}} \cdot \left(1 - \frac{N}{N_f}\right)^{0.260 \cdot t - 0.135} \quad (8)$$

TABLE 6 Four-point bending residual strength of sandstone asphalt mixture considering the influence of water immersion.

Mixture	Stress ratio	Residual strength of four-point bending under different damage Levels (MPa)				
		0%	20%	50%	65%	80%
SBS	0.2	15.885	15.056	13.744	11.879	11.119
	0.3	17.393	16.564	15.118	13.392	12.494
	0.4	18.549	17.652	16.339	14.543	13.369
PM + SBS	0.2	16.277	15.670	14.710	13.345	12.789
	0.3	17.830	17.223	16.164	14.900	14.243
	0.4	19.021	18.364	17.402	16.087	15.227
SBS (Immersion)	0.2	15.133	13.632	11.258	7.882	6.507
	0.3	16.570	15.069	12.452	9.327	7.703
	0.4	17.672	16.048	13.671	10.421	8.296
PM + SBS (Immersion)	0.2	15.552	14.450	12.707	10.229	9.220
	0.3	17.043	15.941	14.017	11.722	10.528
	0.4	18.186	16.992	15.245	12.856	11.294

TABLE 7 Parameters of the fit curve for damage evolution of sandstone asphalt mixture based on residual strength.

Mixture	Stress ratio	u
SBS	0.2	0.235
	0.3	0.217
	0.4	0.208
PM + SBS	0.2	0.160
	0.3	0.148
	0.4	0.143
SBS (Immersion)	0.2	0.531
	0.3	0.483
	0.4	0.459
PM + SBS (Immersion)	0.2	0.341
	0.3	0.313
	0.4	0.299

SBS(Immersion):

$$R_{SBS-I-x\%} = R_{SBS-I} \cdot \left(1 - \frac{N}{N_f}\right)^{0.175 \cdot t - 0.085} \quad (9)$$

SBS + PM:

$$R_{SBS+PM-x\%} = R_{SBS+PM} \cdot \left(1 - \frac{N}{N_f}\right)^{0.599 \cdot t - 0.36} \quad (10)$$

SBS + PM(Immersion):

$$R_{SBS+PM-I-x\%} = R_{SBS+PM-I} \cdot \left(1 - \frac{N}{N_f}\right)^{0.380 \cdot t - 0.21} \quad (11)$$

Where, $R_{SBS-x\%}$ is the residual strength of SBS modified asphalt under x% fatigue life under non immersed conditions, MPa; $R_{SBS-I-x\%}$ is the residual strength of SBS modified asphalt under x% fatigue life under immersion conditions, MPa; $R_{SBS+PM-x\%}$ is the residual strength of SBS + PM asphalt under x% fatigue life under non immersed conditions, MPa; $R_{SBS+PM-I}$ is the residual strength of SBS + PM asphalt under x% fatigue life under immersion conditions, MPa.

Under non-water immersed conditions, the damage factor of the PM-modified sandstone asphalt mixture demonstrates a marginal reduction compared to the control mixture without anti-stripping agent. When subjected to water immersion, however, the damage factor exhibits a substantial percentage decrease in amplitude. These findings indicate that the PM anti-stripping agent significantly enhances the water stability of sandstone asphalt mixtures and effectively mitigates the degradation of fatigue residual strength under combined hydro-mechanical loading conditions. The u -value serves as an indicator of damage susceptibility during fatigue loading, with higher values corresponding to accelerated decay rates in mixture performance. Comparative analysis reveals that the SBS + PM modified sandstone asphalt mixture shows a 31.6%

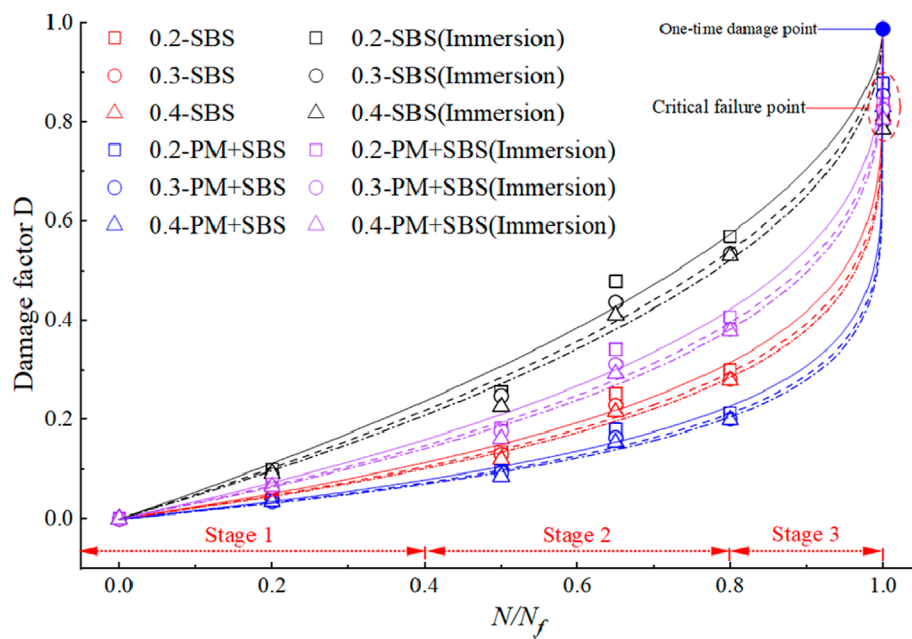


FIGURE 9
Damage evolution law of residual strength of sandstone asphalt mixture considering the effect of water immersion.

reduction in u -value under non-water immersed conditions and a 38.7% decrease under water immersed conditions relative to the conventional SBS-modified mixture. This demonstrates that the incorporation of PM-type anti-stripping agent effectively improves the fatigue strength degradation resistance of sandstone asphalt mixtures, thereby enhancing their long-term durability.

4 Conclusion

To address the insufficient moisture stability between sandstone aggregates and asphalt binders, this study incorporated PM-type anti-stripping agent into SBS-modified sandstone asphalt mixtures. The improvement effects of the anti-stripping agent on strength and fatigue performance were systematically investigated through asphalt and mixture tests. The main conclusions are as follows:

- (1) The experimental results obtained from the three fundamental asphalt indices and boiling water adhesion testing revealed that both XT and PM anti-stripping agents induced no statistically significant alterations in the penetration or softening point characteristics of SBS-modified asphalt. However, they substantially enhanced the ductility performance and adhesion coefficient. Comparative analysis demonstrated that the PM-type anti-stripping agent exhibited superior efficacy in augmenting the interfacial adhesion properties of asphalt relative to its XT counterpart. At an optimal 0.4% PM anti-stripping agent dosage, the SBS-modified asphalt system yielded a marked increase in dynamic viscosity while achieving a superior adhesion classification (Grade ≥ 4) between the asphalt binder and sandstone aggregates, as quantified through standardized adhesion grading protocols.
- (2) The dynamic compressive strength of sandstone asphalt mixtures exhibited a power-law dependence on loading rate under both water immersed and non-water immersed conditions, characterized by an initial rapid increase followed by gradual asymptotic stabilization. Comparative analysis revealed that the PM + SBS composite mixture demonstrated a 5.3% enhancement in dynamic strength relative to the conventional SBS-modified mixture under non-water immersed conditions, with this improvement amplifying to 8.4% under water immersed conditions.
- (3) The PM anti-stripping agent effectively enhanced the post-water immersion fatigue performance of sandstone asphalt mixtures. Adding PM agent increased fatigue life by 33.8% in non-water immersed mixtures and 38.9% under water immersion. Compared to the SBS-modified sandstone asphalt mixture, the SBS + PM composite exhibited a 22.1% increase in the fatigue resistance coefficient (k -value) and a 19.8% reduction in the stress sensitivity index (n -value) under non-water immersed conditions. Under water immersed conditions, these enhancements were further amplified, with the (k -value) increasing by 25.3% and the (n -value) decreasing by 23.1%.
- (4) Considering the combined effects of water immersion and stress, the damage evolution curves of sandstone asphalt mixtures based on residual strength exhibit a three-phase evolution pattern. Under non-water immersed conditions, the in-corporation of a PM-type anti-stripping agent induces a slight decrease in both damage factor and damage sensitivity index compared to SBS-modified sand-stone asphalt mixtures. In water immersed environments, however, these parameters demonstrate a marked reduction in magnitude,

indicating that the water immersion fatigue resistance is significantly enhanced.

Data availability statement

The original contributions presented in the study are included in the article/supplementary material, further inquiries can be directed to the corresponding author.

Author contributions

DZ: Conceptualization, Writing – original draft. ZhZ: Methodology, Writing – review and editing. TH: Writing – review and editing, Conceptualization. ZH: Software, Writing – review and editing. ZiZ: Data curation, Writing – review and editing. JH: Project administration, Funding acquisition, Validation, Writing – review and editing.

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Conflict of interest

Author DZ was employed by Guangxi GuiTong Engineering Management Group Co., Ltd. Author JH was employed by Guangxi Beitou Low-altitude Economy Investment Co., Ltd.

The remaining authors declare that the research was conducted in the absence of any commercial or financial relationships that could be construed as a potential conflict of interest.

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